GEOTECHNICAL INVESTIGATION NEW MODULAR CLASSROOM BUILDING AT THE PHOENIX HIGH SCHOOL SITE AND STADIUM CONCESSION BUILDING SANTA TERESA HIGH SCHOOL CAMPUS 6150 SNELL AVENUE SAN JOSE, CALIFORNIA

for

East Side Union High School District Attn: Linda da Silva 830 N. Capitol Avenue San Jose, California 95133

by

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December 9, 2015 Project No. 978.17H Ser. 4943

East Side Union High School District Attn: Ms. Linda da Silva 830 North Capitol Avenue San Jose, CA 95133-1398

RE: GEOTECHNICAL INVESTIGATION

NEW MODULAR CLASSROOM BUILDING AT THE PHOENIX HIGH

SCHOOL SITE AND STADIUM CONCESSION BUILDING

SANTA TERESA HIGH SCHOOL CAMPUS

6150 SNELL AVENUE SAN JOSE, CALIFORNIA

Dear Ms. da Silva:

As authorized, we have performed a geotechnical and geohazard investigation for the planned new modular classroom building project at the Phoenix High School site and the new stadium concession building on the Santa Teresa High School Campus in San Jose, California. The accompanying report presents the results of our field investigation, laboratory testing and engineering analyses. The site subsurface conditions are discussed, and recommendations for the soil and foundation engineering aspects of the project design are presented. recommendations presented in this report are contingent upon our review of the grading and foundation plans and observation/testing of the earthwork and foundation installation phases of the construction.

Please refer to the text of the report for detailed findings and recommendations. If you have any to questions concerning the report, please call.

Very truly yours,

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INTRODUCTION

This report presents the results of our geotechnical investigation for the planned new modular classroom building project at the Phoenix High School site and the new concession building at the stadium on the Santa Teresa High School Campus in San Jose, California. The approximate location of the project site is shown on the Site Vicinity Map, Drawing 1. The purpose of this investigation was to explore the soil and foundation conditions in the general location of the planned new building projects and develop recommendations for the geotechnical engineering aspects of the project design. Our investigation has also included an analysis of potential geologic hazards associated with the sites.

As shown of the site plan provided to us by Gilbane December 2, 2015, the new classroom building project will include the construction of a one-story modular building (45 foot by 45 foot) in the parking lot/basketball court at the Phoenix High School site and a new concession building (25 foot by 35 foot) on the southeast corner of the stadium/track. The project will also include installation of associated underground utilities.

Our firm has provided geotechnical investigations and construction observation and testing services for the following projects at Santa Teresa High school:

- · Sports Facility Improvements (2004)
- · Multi-Purpose Classroom Building (2009)
- · Swimming Pool Facility (2014 construction pending)

SCOPE

As outlined in our proposal agreement dated September 29, 2015, the scope of our services for this investigation has included:

A. Geotechnical Investigation

- 1. Several reconnaissances of the site by our staff and review of relevant published and unpublished geologic literature and maps.
- 2. A subsurface investigation including the drilling and sampling of five (5) borings in the general area of the planned new buildings.
- 3. Engineering analysis of the field and laboratory data.
- 4. Preparation of this geotechnical investigation and geologic and seismic hazards assessment report for use in the project design and construction. The report includes findings and recommendations for the following:
 - a. Geologic and seismic setting of the site and surrounding area, including research and review of available geologic/seismic reports and maps.
 - b. 2013 CBC seismic design criteria.
 - c. Site preparation including fill placement and grading.
 - d. New building foundation type, and applicable soil and foundation engineering design criteria.

- e. Estimated foundation settlements.
- f. Support of interior and exterior concrete slabs-on-grade (including subgrade/pad preparation).
- g. Treatment of expansive soils.
- h. Backfilling and compaction of utility trenches.
- i. Flexible pavements.
- j. Any other unusual design or construction conditions encountered in the investigation.

B. Geologic and Seismic Hazards Assessment

The Geologic and Seismic Hazards Assessment portion of our report for the project consists of the following:

- 1. Discussion of geologic and seismic conditions and data on the nature of the site and potential earthquake damage including:
 - a. Regional geology and seismic conditions and historical information on the seismicity of the local and regional area.
 - b. Location of known active and potentially active faults near the site, as well as nearby inactive faults.

- Earthquake ground motion acceleration design parameters and geologic site classification in accordance with the 2013 California Building Code study requirements.
- Potential site impacts related to faulting, liquefaction, lateral spreading, ground cracking, seismic settlement and differential compaction, landsliding, flooding, and dam failure inundation with recommended mitigation measures, where appropriate.

This report has been prepared for the specific use of the East Side Union High School District and its consultants in accordance with generally accepted geotechnical engineering principles and practices. No other warranty, either expressed or implied, is made. In the event that any substantial changes in the nature of the project are planned, the conclusions and recommendations of this report shall not be considered valid unless such changes are reviewed and the conclusions of this report modified or verified in writing. Any use or reliance of this report or the information herein by a third party shall be at such party's sole risk.

It should also be recognized that changes in the site conditions may occur with the passage of time due to environmental processes and/or acts of man, and that changes in building codes, the state of the practice or new information may require modifications in the recommendations presented herein. Accordingly, neither the client, nor any other party should rely on the information or conclusions contained in this report after three years from its date of issuance without the express written consent of Cleary Consultants, Inc.

METHOD OF INVESTIGATION

A site reconnaissance and the subsurface investigation were performed on October 25, 2015, using a truck-mounted hollow-stem auger drill rig. Five exploratory borings were drilled under

the guidance of our staff engineering geologist, Dustin Lettenberger to a maximum depth of 45.0 feet at the locations shown on Drawings 4 and 5. A key describing the soil classification system and soil consistency terms used in this report is presented on Drawing 6 and the soil sampling procedures are described in Drawing 7. Logs of the borings are presented on Drawings 10 through 18.

The borings were located in the field by pacing/tape measurements and interpolation of the features shown on the site plan provided us. These locations should be considered accurate only to the degree implied by the method used.

Samples of the soil materials from the borings were returned to our laboratory for classification and testing. The results of moisture content, dry density, percent finer than No. 4 and No. 200 sieves, plasticity index and free swell testing are shown on the boring logs. The laboratory testing procedures followed during this investigation are summarized on Drawings 8 and 9. Drawing 19, Plasticity Chart, present additional data on the plasticity index testing. Drawings 20 and 21 present the results of R-Value testing performed on an untreated and a chemically treated sample of the upper soils. The results of soil corrosivity testing performed on a composite sample of the surficial soils collected from the borings are presented on Drawing 22.

A list of references consulted during the investigation is included at the end of the text.

SITE CONDITIONS

A. Surface

The planned modular classroom building site is located within an existing relatively level asphalt paved basketball court and adjacent parking lot, on the southwest side of the main campus.

The planned concession building site is located in an existing relatively level asphalt paved area at the southeast corner of the track/stadium, on the east side of the main campus. The site is bordered by a grass softball field to the south and grass playfield to the east.

Small trees are located north of the modular classroom building site and south of the concession building site.

The average elevation of the campus is approximately 166 feet M.S.L. The overall regional topographic gradient is approximately 0.2 percent to the west on the property.

B. Subsurface

The exploratory borings at the two sites generally encountered stiff to very stiff sandy clay and silty clay to the maximum depth explored of 45 feet. EB-1 and EB- encountered a layer of loose to medium dense clayey sand in the upper one and one-half feet. EB-4 encountered a layer of dense gravelly silty sand from approximately 32 feet to 42 feet.

The upper sandy clay and silty clay soils encountered in EB-1 through EB-5 are considered to have a moderate to high expansion potential based on their plasticity characteristics (Plasticity Indices = 27 to 40 percent) and the free swell test data (Free Swells of 35 to 80 percent).

The attached boring logs and related information depict subsurface conditions only at the specific locations shown on Drawing 4 and on the particular date designated on the logs. Soil conditions at other locations may differ from conditions occurring at these boring locations. Also, the passage of time may result in a change of soil conditions at these boring locations due to environmental changes.

C. Groundwater

Groundwater was encountered in EB-4 during drilling at a depth of approximately 37 feet; groundwater was not encountered in the other borings drilled for this investigation. It should be noted that the borings were only open for a period of a few hours and this may not have been sufficiently long to establish the stabilized water table conditions. It should also be noted that fluctuations of localized perched groundwater and the regional groundwater level can occur due to such factors as variations in rainfall, temperature, runoff, irrigation, and other factors not evident at the time our measurements were made and reported herein.

The historically high groundwater level in the site vicinity has been mapped at a depth of approximately 10 feet below the ground surface on Seismic Hazard Report 097, Plate 1.2, "Depth to Historically High Groundwater" for the Santa Teresa Hills Quadrangle. We have therefore conservatively assumed a groundwater depth of 10 feet for liquefaction analysis.

GEOLOGY AND SEISMICITY

The school site is located at the south end of the Santa Clara Valley, a broad, sediment filled basin bordered on the east by the Diablo Range and on the west by the Santa Cruz Mountain Range. Structurally, the Santa Clara Valley has formed as a result of tectonic downwarping controlled by three northwest trending active fault zones: the San Andreas fault on the southwest and the Hayward and Calaveras faults on the northeast. The site is shown to be underlain by alluvial fan deposits (Qa) as indicated on published geologic mapping of this area (Dibblee, 2005) as shown on Drawing 2, Local Geologic Map.

The San Francisco Bay Area is recognized by geologists and seismologists as one of the most active seismic regions in the United States. The three major fault zones which pass through the Bay Area in a northwest direction have produced approximately a dozen earthquakes per century

strong enough to cause structural damage. The faults causing these earthquakes are part of the San Andreas fault system, a major rift in the earth's crust that extends for at least 450 miles along the California Coast and includes the Calaveras, Northern San Andreas, and Hayward-Rodgers Creek faults. The site is located approximately 9.0 miles southwest of the Calaveras fault, 9.6 miles northeast of the San Andreas fault, and 15.2 miles southwest of the Hayward fault, respectively. In addition, the site is located about 1.8 miles northeast of the potentially active Monte Vista-Shannon fault.

Since the early 1800's, major earthquakes have been recorded along the San Andreas, Hayward and Calaveras fault zones (Toppozoda et al, 2000). In 1861, an earthquake having a Richter magnitude of approximately 6.5 was reported on the Calaveras fault. The presumed epicenter of this earthquake was located approximately 36 miles northwest of the site. In 1984, an earthquake having a Richter magnitude of approximately 6.1 was reported on the Calaveras fault near Mt. Hamilton. The epicenter of this earthquake was located approximately 10 miles northeast of the site. In 1868, an earthquake having an estimated Richter magnitude of 7.0 was recorded along the Hayward fault. This earthquake opened fissures at random locations along the fault, from San Pablo to Mission San Jose. The presumed epicenter of the 1868 earthquake is located approximately 35 miles northwest of the site. The San Francisco Earthquake of 1906 had a Richter magnitude of approximately 8.3 and the epicenter of this earthquake (Toppozoda et al, 2000) was located approximately 49 miles northwest of the site; also, the San Andreas fault produced earthquakes having approximate magnitudes of 7.0 and 6.6 in 1838 and 1865, the presumed epicenters of which are located about 18 miles northwest and 5 miles southwest of the site.

An earthquake with Richter magnitude 5.4 experienced on the Concord fault in 1955 had its epicenter approximately 52 miles northwest of the site. Another damaging earthquake with Richter magnitude 5.3 occurred in 1957 on the San Andreas fault in Daly City, causing approximately one million dollars in damage. The epicenter of this earthquake was about 49 miles northwest of the site. Two earthquakes in 1980, along traces of the Greenville fault, had

their epicenters approximately 41 miles northeast of the site. These 1980 earthquakes had Richter magnitudes of 5.5 and 5.8. In addition, numerous earthquakes of magnitudes 4.0 or greater have been recorded throughout the Bay Area along the San Andreas, Hayward and Calaveras faults.

On October 17, 1989, the Loma Prieta earthquake, which had its epicenter 14 miles southwest of the school site and a Moment Magnitude of 6.9, produced widespread damage through the Bay Area. Damage in the San Jose area was relatively light, however, with a shaking intensity of Modified Mercalli Intensity VII (damage negligible in buildings of good design and construction) and recorded peak horizontal accelerations in the range of 0.11 to 0.13g. Most of the liquefaction-related damage caused by this earthquake occurred in areas of shallow water table (10 feet or less) underlain by unconsolidated fill and loose soil deposits, such as the Marina District of San Francisco, the westerly portion of Oakland, and downtown Santa Cruz.

On August 24, 2014, a Magnitude 6.0 earthquake occurred in the vicinity of the West Napa fault near American Canyon in Napa County; this earthquake, which had its epicenter approximately 73 miles northwest of the site, caused extensive damage in south Napa County.

The distances between the site and the capable segments of the above faults, as well as other significant faults within a radius of 60 miles from the site, was determined using the USGS Earthquake Hazards Program 2008 USGS National Seismic Hazard Maps – Fault Parameters, as presented below in Table 1:

TABLE 1 - Summary of Significant Earthquake Faults Capable of Generating
Strong Ground Shaking at the new Classroom and Concession Building sites, Santa
Teresa High School (1), (2)

Earthquake Generating Fault	Approximate Distance and Direction to Generating Fault (miles)	Maximum Earthquake (Moment Magnitude)	
Monte Vista - Shannon	1.8 SW	6.5	
Calaveras CN+CC+CS	9.0 NE	7.0	
Northern San Andreas SAO+SAN+SAP+SAS	9.6 SW	8.1	
Zayante-Vergeles	12.9 SW	7.0	
Hayward –Rodgers Creek RC+HN+HS	15.2 NE	7.3	
Greenville Connected	24.1 NE	7.0	
Monterey Bay-Tularcitos	27.1 SW	7.3	
San Gregorio Connected	27.1 SW	7.5	
Ortigalita	30.2 SE	7.1	
Quien Sabe	33.3 SE	6.6	
Mount Diablo Thrust	34.1 NE	6.7	
Great Valley 7	36.4 NE	6.9	
Rinconada	38.8 SW	7.5	
Green Valley Connected	46.8 NE	7.0	

⁽¹⁾ USGS Earthquake Hazards Program 2008 USGS National Seismic Hazard Maps – Fault Parameters, December 2, 2015

The historical seismicity of the greater San Francisco Bay Area and surrounding region is presented on Drawing 3, Regional Earthquake Epicenter Map.

Similar to most of the San Francisco Bay Area, it is reasonable to assume that the new modular classroom and concession buildings will be subjected to a moderate to severe earthquake from one of the above-mentioned faults during their lifetime. During such an earthquake, strong ground shaking is likely to occur at the sites.

⁽²⁾ Site Latitude: 37.2350°N; Site Longitude: 121.8279°W

GEOLOGIC AND SEISMIC HAZARDS EVALUATION

A. Fault Offset Hazards

Based on our site reconnaissance, field exploration and review of existing geologic information, we conclude that there are no known active or potentially active faults crossing the school sites.

The sites are not located within an Alquist-Priolo Earthquake Fault Zone as defined by the State of California or in a fault hazard zone of the City of San Jose or Santa Clara County. Based on the above, it is our opinion that the hazard resulting from surface rupture or fault offset at the sites is low.

B. Ground Shaking Hazards

1. Strong Ground Shaking

Strong ground shaking is likely to occur during the lifetime of the planned new buildings as a result of movement along one or more of the regional active faults discussed above. The new buildings will need to be designed and constructed in accordance with current standards of earthquake-resistant construction.

Ground shaking during an earthquake could cause furnishings which are not rigidly attached (such as bookshelves and file cabinets) to undergo movement with respect to the building. Design measures that minimize such potential movement and also minimize the adverse effects of such movement where they cannot be prevented should be utilized.

2. Soil Liquefaction

Liquefaction is a phenomenon in which saturated cohesionless soils lose strength during strong shaking and experience horizontal and vertical movements. Soils that are most susceptible to liquefaction are clean, loose, saturated, uniformly graded, fine-grained clay-free sands and silts that lie within 50 feet of the ground surface.

Our investigation found that the planned new building sites are predominantly underlain by stiff to very stiff sandy clay and silty clay (with fines contents of 72 percent to 100 percent) to the maximum depth explored of 45 feet. EB-1 and EB-4 were analyzed for potential liquefaction-induced settlement using the LiquefyPro computer program (Version 5.0) and a factor of safety (FOS) of 1.3 per CGS Special Publication 117A. The assumed groundwater depth used in the analysis was 10 feet (historic high level). One-inch blowcounts were recorded in the sand layers encountered during drilling in intervals where gravels were observed, however, gravel interference was not indicated.

LiquefyPro evaluates liquefaction potential and calculates the settlement of saturated and unsaturated deposits due to seismic loads using SPT blowcount, total unit weight, fines content, peak horizontal acceleration and earthquake moment magnitude data. The program is based on the most recent publications of the NCEER Workshop and SP117 Implementation.

The fine grained sandy clay and silty clay layers were further analyzed for liquefaction susceptibility using criteria from Bray, J.D. and Sancio, R.B. in their 2006 paper "Assessment of the Liquefaction Susceptibility of Fine Grained Soils". This study found that fine grained soils with a plasticity index of 12 or more and a water content to liquid limit ratio of less than 0.8 are not susceptible to liquefaction. Based on these criteria, the sandy clay and silty clay layers encountered at the site were not found to be susceptible to liquefaction.

Based on the results of our analysis, theoretical liquefaction-induced settlement of up to one-quarter inch total could occur at the sites, with one-eighth inch predicted differential settlement over a distance of 50 feet, using the calculated peak ground acceleration ($PGA_M = 0.735$) for the sites as specified in Item 20 of CGS Note 48 (2013), and the Tokimatsu and Seed calculation method with magnitude scaling correction. The results and supporting data for the liquefaction analysis are included in Appendix A of this report.

The sites are mapped within an area of regional liquefaction potential (State of California Seismic Hazard Zones Map, Santa Teresa Hills Quadrangle, August 14, 2003). However, based on the above information, we conclude that the likelihood that the planned improvements will be damaged by earthquake-induced soil liquefaction is very low.

3. Soil Densification

The recognized procedures for evaluation of seismically-induced settlement in dry sandy soils (Tokimatsu and Seed, 1987; Pradel, 1998) are considered most applicable to non-cohesive loose clean sands with less than 5 percent fines (Day, 2002). The silty clay layers encountered at the site are not considered to be susceptible to soil densification due to their predominantly high fines content, however, these layers including the gravelly clayer sand layer encountered in EB-1 were analyzed for potential soil densification.

The analysis indicates that the theoretical seismically-induced settlement is nil at the sites.

Based on the above information, we conclude that the likelihood that the new buildings and improvements will be damaged by earthquake-induced soil densification is remote.

4. Other Seismic Hazards

We have also considered the possibility of other seismically induced hazards at the sites including lateral spreading in the direction of a free-face. The Canoas Creek Channel is located approximately 150 feet south of the classroom site and 100 feet south of the concession building site. The unlined channel is about ten feet deep with approximately 2:1 (horizontal to vertical) side slopes. Due to the stiff to very stiff consistency and high fines content (72 to 100 percent) of the silty clay and sandy clay layers encountered to a depth of 45 feet in EB-1 and EB-4, it is our opinion that any lateral displacement associated with the creek during a seismic event would most likely result in localized sloughing along the creek banks rather than areal movement.

Ground cracking may be caused by any of the phenomena discussed above. Since there is a low potential for liquefaction, soil densification and lateral spreading of the soils underlying the sites, it is also considered unlikely that significant ground cracking will occur. Also, the potential for landsliding at the sites is very low due to their essentially level states.

C. Flooding

Fema Flood Insurance Mapping (2009) indicates the Santa Teresa High School campus to be within Zone D, "areas in which flood hazards are undetermined, but possible". The Base Flood Elevation (BFE) is not provided as the site is not located in Flood Hazard Zone A or E. The southern side of the campus bordered by Canoas Creek Channel is indicated as Zone A, an area of "special flood hazard areas subject to inundation by the one percent annual chance flood."

The 2003 Flood Inundation Maps prepared by the Santa Clara Valley Water District for Anderson Reservoir, located southeast of the school in the Diablo Range, indicate that the sites, similar to much of San Jose, is within the "fair weather" and "IDF" (catastrophic failure of the reservoir when filled to capacity, combined with a large storm inflow) inundation events. The likelihood of such an event, however, is considered very low.

Based on the Santa Clara County Water District Maps (dated January 1996), the school is mapped within the inundation zone resulting from catastrophic failure of Calero Dam, located southeast of the site. However, the likelihood of such an event is also very low.

Santa Clara County Water District Inundation Maps for the Guadalupe Dam (dated September 1973), the Coyote Percolation Pond (dated March 1974) and Almaden Dam (dated September 1973) indicate that site lies outside the mapped inundation zones of these three dams.

CONCLUSIONS AND RECOMMENDATIONS

Based on the findings of our investigation, we judge that there are no geologic hazards or constraints which would preclude the construction of the proposed new modular classroom building and concession building projects at the Santa Teresa High School campus. From a soil and foundation engineering standpoint, we also conclude that the improvements can be constructed as planned provided the recommendations of this report are incorporated into the design and construction of the project.

The primary geotechnical constraint at the site is the moderately to highly expansive near-surface silty clay soil, which could undergo significant shrink-swell movement as a result of changes in the soil moisture content. Therefore, it will be necessary to properly moisture condition (to at least two percent above optimum) and recompact the upper 12 inches of surface soils beneath slabs, and support the new structures on deepened spread footings which obtain support below

the zone of seasonal moisture change in the stiff to very stiff clay and/or in properly compacted engineered fill. The proposed slabs-on-grade will also need to be supported on a cushion of imported, non-expansive material, such as Class 2 aggregate base, to minimize slab movements due to long-term moisture fluctuations; and it will also be necessary to keep the soils underlying the cushion in a moist state prior to placing the non-expansive materials.

Our analysis indicates that the potential total seismically-induced dry soil settlement at the site is negligible and the maximum theoretical liquefaction-induced settlement is approximately one-quarter inch, with one-eighth inch theoretical differential settlement over a 50 foot span. The new buildings should be designed to accommodate potential movements of this magnitude.

The recommendations presented in the remainder of this report are contingent on our review of the earthwork and foundation plans for the project and our observation of the grading, foundation installation, concrete slab and pavement installation phases of the construction.

A. Earthwork

1. Stripping and Site Preparation

Existing pavements, underground utilities, curbs, underground obstructions and other site improvements (not designated to remain) within areas to be graded should be removed to their full depth and extent and hauled from the site. Any pre-existing fills encountered within these areas should be removed to their full depth and extent.

The new construction areas should then be stripped to a sufficient depth to remove all pavements, debris-laden soils, surface vegetation and organic rich topsoil.

Holes resulting from the removal of underground obstructions (such as abandoned utilities) or pre-existing fills that extend below the planned finished grade should be cleared of loose soil and debris, moisture conditioned and recompacted, and backfilled with suitable material compacted to the requirements discussed below for engineered fill (see Section 3, Fill Placement and Compaction).

2. Moisture Conditioning and Recompaction of Surface Soils

After the new construction areas have been properly prepared and required excavations have been made, the surface soils in new building, pavement and sidewalk areas, including any areas to be filled, should be properly moisture conditioned and recompacted. This work should consist of ripping the upper 12 inches, thoroughly moisture conditioning the soils to at least two percent above optimum moisture content, and compacting them to at least 90 percent relative compaction as determined by ASTM Test Designation D1557. Compaction should be performed using appropriately sized compaction equipment such as a self-propelled sheepsfoot compactor. Any required additional fill then can be placed after the surface soils have been scarified, moisture conditioned, and recompacted. The moisture conditioned soils should not be allowed to dry out prior to placing new fill.

Any unstable or pumping subgrade areas should be chemically treated, or subexcavated, plugged with baserock and overlain with a stabilizing fabric such as Mirafi 600X. Fabric installation should be performed in accordance with the manufacturer's recommendations. The method and extent of any required stabilization work should be evaluated by our representative.

3. Fill Placement and Compaction

Existing soils having an organic content of less than three percent by volume and which are free of construction debris can be used as fill. Fill material should not, however, contain rocks or lumps greater than six inches in greatest dimension with not more than 15 percent larger than 2.5 inches. Any imported fill required to raise grades in the building, sidewalk and pavement areas should be predominantly granular with a maximum plasticity index of 15.

Engineered fill should be compacted to at least 90 percent relative compaction as determined by ASTM Test Designation D1557. Fill material should be spread and compacted in lifts not exceeding eight inches in uncompacted thickness. The moisture content of both on-site and imported soils utilized as fill should be adjusted at least two percent above their optimum moisture content.

In order to achieve satisfactory compaction in the subgrade and fill soils, it may be necessary to adjust the soil moisture content at the time of soil reworking. This may require that water be added and thoroughly mixed into any soils which are too dry or that scarification and aeration be performed in any soils which are too wet. The subgrade will require rescarification and recompaction if it is allowed to dry out and crack prior to placing the required non-expansive material section.

4. Trench Backfill

The presently available subsurface information indicates that the required utility trenches can be excavated with conventional backhoe equipment. Any trenches deeper than five feet should be properly braced or sloped in accordance with the current requirements of CAL-OSHA or the local governmental agency, whichever is more stringent.

Utility trenches should be backfilled with engineered fill placed in lifts not exceeding eight inches in uncompacted thickness, except thicker lifts may be used with the approval of the soil engineer provided satisfactory compaction is achieved. If on-site soils are used, the materials should be compacted to at least 85 percent relative compaction by mechanical means only. Imported sand also can be used for backfilling trenches provided it is compacted to at least 90 percent relative compaction. In slab and pavement areas, the trench backfill should be compacted to at least 90 percent relative compaction for on-site soils, and 95 percent where imported clean sand backfill is used.

Water jetting of the trench backfill as a means to achieve the required compaction should not be permitted.

5. Surface Drainage

Positive surface gradients of at least two percent on porous surfaces and one percent on paved surfaces should be maintained away from the buildings and other planned improvements so that water does not pond in the vicinity of the foundations. Any collected runoff should be carried away from the improvements and discharged into approved drainage facilities or onto hardscape surfaces which drain toward collection basins or surface collectors.

6. Construction Observation

Grading and other earthwork-related operations should be observed and tested by our representative for conformance with the project plans/specifications and our recommendations. This work includes site preparation, selection of satisfactory fill materials, and placement and compaction of the subgrades and fills. Sufficient notification prior to commencement of earthwork is essential to make certain that the work will be properly observed.

B. Modular and Concession Building Foundations

After the site has been properly prepared and graded, the new building loads can be supported on deepened continuous and isolated spread footings which bear in the undisturbed stiff silty clay soils encountered at the site or in properly engineered fill.

Footings should be founded at least 30 inches below lowest adjacent finished grade and embedded at least 18 inches into the supporting subgrade soil. Continuous footings should have a minimum width of 18 inches and isolated footings should be at least 24 inches square. Footings located adjacent to utility trenches should have their bearing surfaces below an imaginary 1.5:1 (horizontal to vertical) plane projected upward from the edge of the bottom of the trench. Care should be taken to keep the footings moist by spraying lightly prior to the concrete pour.

At the above depths, building footings can be designed for an allowable bearing pressure of 2000 psf due to dead loads with a one-third increase for dead plus live loads (2667 psf) and a 50 percent increase for total design loads (3000 psf) including wind and seismic. All continuous footings should be provided with at least two number four reinforcement bars top and bottom, to provide structural continuity and to permit spanning of local irregularities.

Our firm should observe the footing bottoms prior to placing reinforcing steel and concrete. Loose soil encountered in the footing bottoms should be removed and replaced as engineered fill, or alternatively, densified by mechanical means (jumping wacker). Any backfill placed beneath the foundations should be compacted to at least 90 percent relative compaction as determined by field density testing.

Lateral loads may be resisted by friction between the foundation bottoms and the supporting subgrade. A friction coefficient of 0.25 is considered applicable. As an alternative, an equivalent

fluid pressure of 250 pcf starting one-half foot below the ground surface can be taken against the sides of footings poured neat.

Utility sleeves and any utility trenches through or beneath the perimeter foundations of the building should be plugged to minimize moisture infiltration beneath the structure. The annular space between the utility pipe and the "sleeve" should be filled with a flexible, waterproof compound manufactured to permanently adhere to both the sleeve material and the particular type of pipe being used in order to reduce both the potential for water seeping beneath the building, as well as for the pipe to be broken at this location during seismic shaking.

Settlements under the anticipated loads are expected to be within tolerable limits for the proposed construction.

C. Seismic Design Parameters

Seismic design values for the project were determined using the online USGS Seismic Design Maps Tool (ASCE 7-10), and the subsurface information obtained from the exploratory borings which was used for determining the site classification. Based on the results, a site-specific seismic hazard analysis is not required (per CBC 2013 Section 1613A.3.5) for the building locations (Site Class D), as $S_1 < 0.75$ and the sites are assigned to Seismic Design Category D.

Using Latitude (37.2350 °N) and Longitude (121.8279 °W) and the site classification as input, the computer application provides mapped acceleration parameters, site coefficients and design spectral acceleration parameters for both "short" (0.2 seconds) and long period (1-second) durations as detailed in the 2013 CBC.

Based on the results of our investigation, the tables provided in Section 1613A.3.3 and 1613A.3.5 of the 2013 CBC, and our analysis using the USGS Seismic Design Maps Tool

(ASCE 7-10), the following seismic design parameters can be used in lateral force analyses for

the sites:

Site Class D - Stiff Soil Profile with Standard Penetration Test Values of 15 to 50 blows/foot

Site Coefficient Fa = 1.0

Site Coefficient Fv = 1.5

Mapped Spectral Acceleration Values; $S_S = 1.912$, $S_1 = 0.653$

Spectral Response Accelerations; $S_{MS} = 1.912$, $S_{M1} = 0.979$

Design Spectral Response Accelerations; $S_{DS} = 1.275$, $S_{D1} = 0.653$

D. <u>Slabs-on-Grade</u>

Slab-on-grade construction will be used for building and exterior slabs. Interior slabs should be

underlain by at least 18 inches of imported virgin Class 2 aggregate baserock placed on the

prepared subgrade soil and provided with a capillary moisture break section, as subsequently

discussed. Exterior concrete flatwork should be underlain by 12 inches (minimum) of Class 2

aggregate baserock placed on the prepared subgrade soil.

Reinforcement of slabs should be provided in accordance with their anticipated use and loading.

but as a minimum, slabs should be reinforced with No. 3 bars at 18 inches on center, both ways,

or No. 4 bars at 24 inches on center, both ways. Concrete slabs should have a maximum control

joint spacing (sawcut or tooled) of 10 feet in both directions. Control joint patterns should

address potentially weak slab areas near cutouts or corners of the slab. We do not recommend

that exterior concrete paving be "hinged" off interior slabs, i.e. exterior slabs should be

constructed independently of building slabs.

Prior to final construction of slabs, the subgrade should be proof rolled to provide a smooth, firm

non-yielding surface. The Class 2 aggregate baserock should be compacted to at least 90 percent

relative compaction.

In building areas where floor wetness would be undesirable, slabs should be underlain by a capillary moisture break section consisting of a minimum 15 mil vapor retarder of permeance less than or equal to 0.01 perms (as tested by ASTM E1249) placed over six inches of 3/4-inch clean, free draining crushed rock. Care should be taken to prevent wear, punctures and/or tearing of the membrane during the construction phase (such as could result from the placement of rebar) subsequent to its installation; any tears or punctures should be tightly sealed. The six inches of 3/4-inch crushed rock can be used in lieu of the upper six inches of the imported Class 2 baserock recommended under building slabs.

All drainrock, baserock and imported material placed beneath interior slabs or within the building pad should be virgin "non-recycled" material.

Prior to final construction of slabs, the subgrade should be proof rolled to provide a smooth, firm non-yielding surface. The moisture content of the compacted subgrade should be maintained at least two percent above optimum moisture prior to placing baserock materials. The subgrade will require rescarification and recompaction if it is allowed to dry out and crack prior to placing the required non-expansive material section.

E. Flexible Pavements

Laboratory testing performed on a sample of the near-surface soils obtained an R-Value of less than five, which would require a relatively thick Class 2 aggregate baserock and asphaltic concrete section to support the anticipated traffic loads at the site. The required thickness of the pavement section can be reduced by chemically-treating the pavement subgrade to a depth of 12 inches with five percent Quicklime Plus (50% Lime/50% Portland cement) which is indicated to provide an R-Value of 73 based on laboratory testing.

Utilizing the preceding untreated and treated R-Values, and assumed Traffic Indices of 4.5 and 6.0 for automobile parking and driveway and bus/fire truck traffic lanes, respectively, and Procedure 301-F of the California Department of Transportation, our analysis indicates the following minimum alternative flexible pavement sections can be used for this project:

Table 2 - Recommended Flexible Pavement Sections

Traffic Condition	Asphaltic Concrete (inches)	Class 2 Aggregate Base (inches)	Quicklime Subgrade Treatment (inches)*	Total Thickness (inches)
Auto Parking				
(T.I. = 4.5)				
Untreated	3.0	8.0		11.0
Chemically Treated*	2.0	4.0	12.0	18.0
Bus/Fire Lane,				
Driveways				
$(\mathbf{T.I.} = 6.0)$				
Untreated	4.0	12.0		16.0
Chemically Treated*	3.0	8.0	12.0	23.0

^{*5%} Quicklime Plus (50% quicklime/50% Portland cement)

The upper six inches of subgrade and the Class 2 aggregate baserock section should be compacted to at least 95 percent relative compaction. Any fill required below the upper six inches of subgrade should be compacted to at least 90 percent.

The subgrade should be statically rolled with a heavy, smooth drum roller to provide a smooth firm surface. Any unstable or pumping subgrade areas should be chemically treated as described above, or subexcavated, plugged with baserock and overlain with a stabilizing fabric such as Mirafi 600X. Fabric installation should be performed in accordance with the manufacturer's recommendations. The method and extent of any required stabilization work should be evaluated by our representative.

AC hardscape pavements should consist of at least two inches of asphaltic concrete over a minimum of six inches of compacted Class 2 aggregate baserock.

Class 2 aggregate base should have an R-Value of at least 78 and conform to the requirements of Section 26, State of California "CALTRANS" Standard Specifications, latest edition. The aggregate base material should be placed in thin lifts in a manner to prevent segregation, and should be uniformly moisture conditioned and compacted to at least 95 percent relative compaction to provide a smooth, unyielding surface.

The asphaltic concrete should conform to and be placed in accordance with the requirements of Section 39 in the State of California CALTRANS Standard Specifications, latest edition.

F. Soil Corrosivity

Laboratory resistivity, pH, chloride and sulfate testing was performed on a composite soil sample obtained from the upper five feet of the borings during our geotechnical investigation for this project. The testing was performed by Cooper Testing Laboratory for the purpose of evaluating the soils' corrosion potential for use in the design of underground utilities and embedded concrete on this project.

In summary, the test results indicated a minimum resistivity of 1,820 ohm-cm, a pH of 8.0, a chloride content of seven ppm, and water soluble sulfate content of 217 ppm. Soils with chloride contents of less than 500 ppm and sulfate contents of less than 1500 ppm are considered to be of "low" corrosivity. However, <u>based on the resistivity testing</u>, the soils are considered "moderately corrosive".

Table 3 below shows the general correlation between resistivity and corrosion potential.

<u>Table 3 - Correlation Between Resistivity</u> <u>and Corrosion Potential (C)</u>

Soil Resistivity (ohm-cm)	Soil Classification		
Below 500	Very Corrosive		
500 to 1,000	Corrosive		
1,000 to 2,000	Moderately Corrosive		
2,000 to 10,000	Mildly Corrosive		
Above 10,000	Progressively Less Corrosive		

(C) National Association of Corrosion Engineers.

This condition combined with the slightly alkaline soil condition encountered at the site could result in reduced life span of steel culverts for this project. Thicker gauge pipelines would have greater life spans. For example, the life spans for 18, 16 and 14 gauge steel culverts with a soil resistivity of 1,313 ohm-cm and a pH of 7.6 are estimated to be roughly 32, 41 and 51 years, respectively (California Division of Highways, 1993).

For the purposes of design of concrete in contact with the soil, there are no restrictions on types of cementitious materials to be used, based on the pH and sulfate testing.

PLAN REVIEW AND CONSTRUCTION OBSERVATION

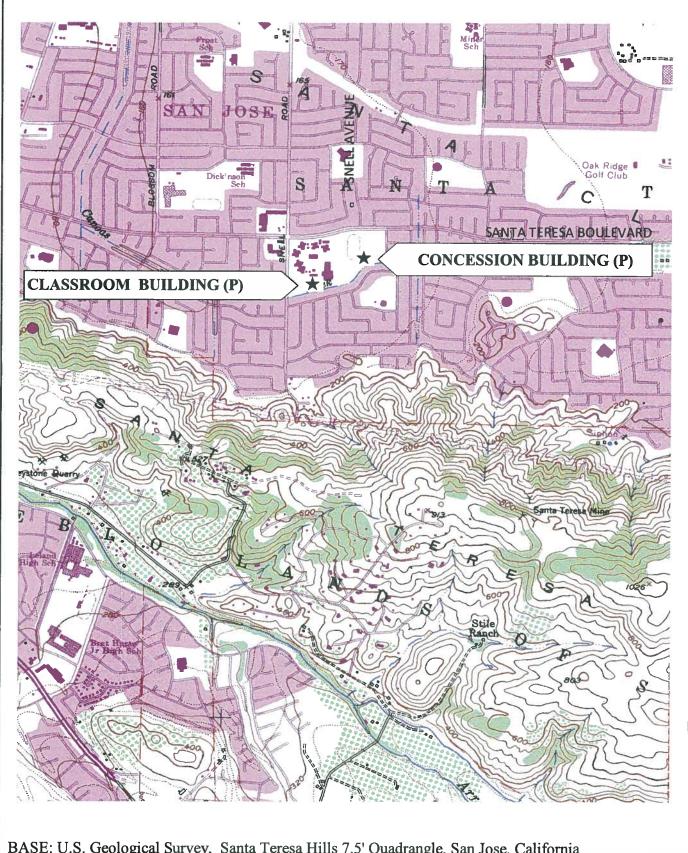
We should be provided the opportunity to review the foundation and grading plans and the specifications for the project when they are available. We should also be retained to provide soil engineering observation and testing services during the grading and foundation installation phases of the project. This will provide the opportunity for correlation of the soil conditions found in our investigation with those actually encountered in the field, and thus permit any necessary modifications in our recommendations resulting from changes in anticipated conditions.

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BASE: U.S. Geological Survey, Santa Teresa Hills 7.5' Quadrangle, San Jose, California
SITE VICINITY MAP

CLEARY CONSULTANTS, INC. Geotechnical Engineers and Geologists APPROVED BY GF NEW CLASSROOM AND CONCESSION BUILDINGS Santa Teresa High School San Jose, California PROJECT NO. DATE DRAWING NO. 978.17H December 2015



EXPLANATION

20 / Strike and Dip of Bedding

Qa Alluvial Fan Deposits

Unnamed Sandstone

Tsh Unnamed Clay Shale

Tss

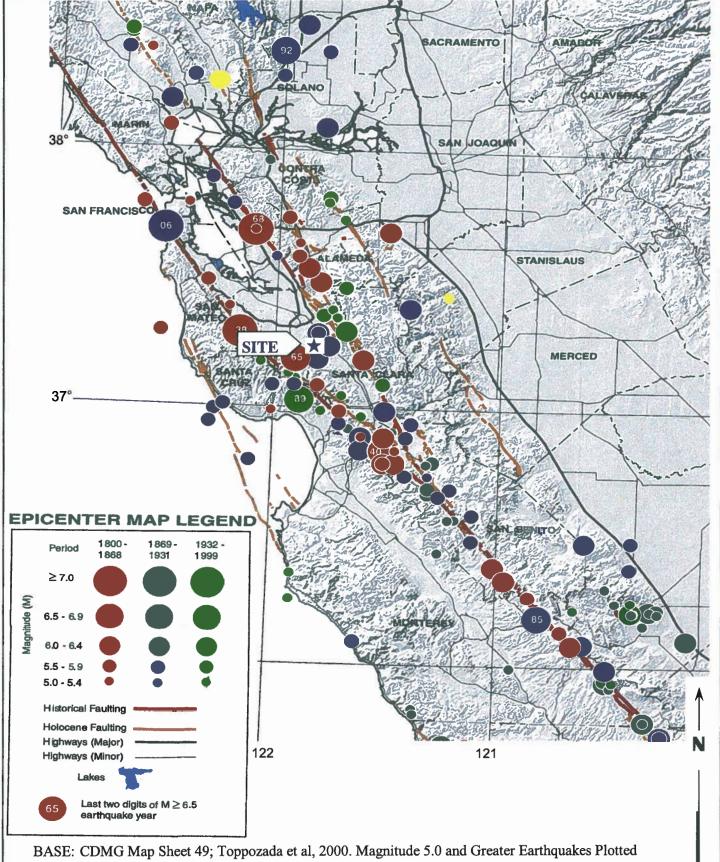
Kps Panoche Formation Sandstone

sp/sc Coast Range Serpentinite

fm Franciscan Melange fc Franciscan Chert

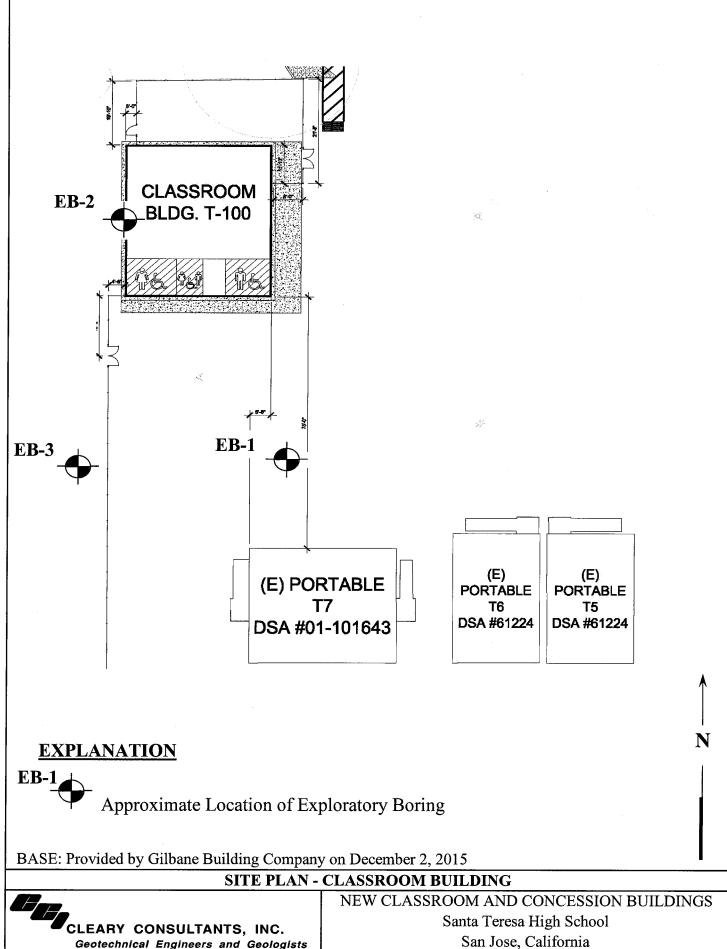
BASE: Thomas Dibblee, Jr., Geologic Map of the Santa Teresa Hills Quadrangle, 2005

LOCAL GEOLOGIC MAP CLEARY CONSULTANTS, INC. NEW CLASSROOM AND CONCESSION BUILDINGS Santa Teresa High School San Jose, California Geotechnical Engineers and Geologists DRAWING NO. APPROVED BY **SCALE** PROJECT NO. DATE GF 1" = 2000' 978.17H December 2015 2

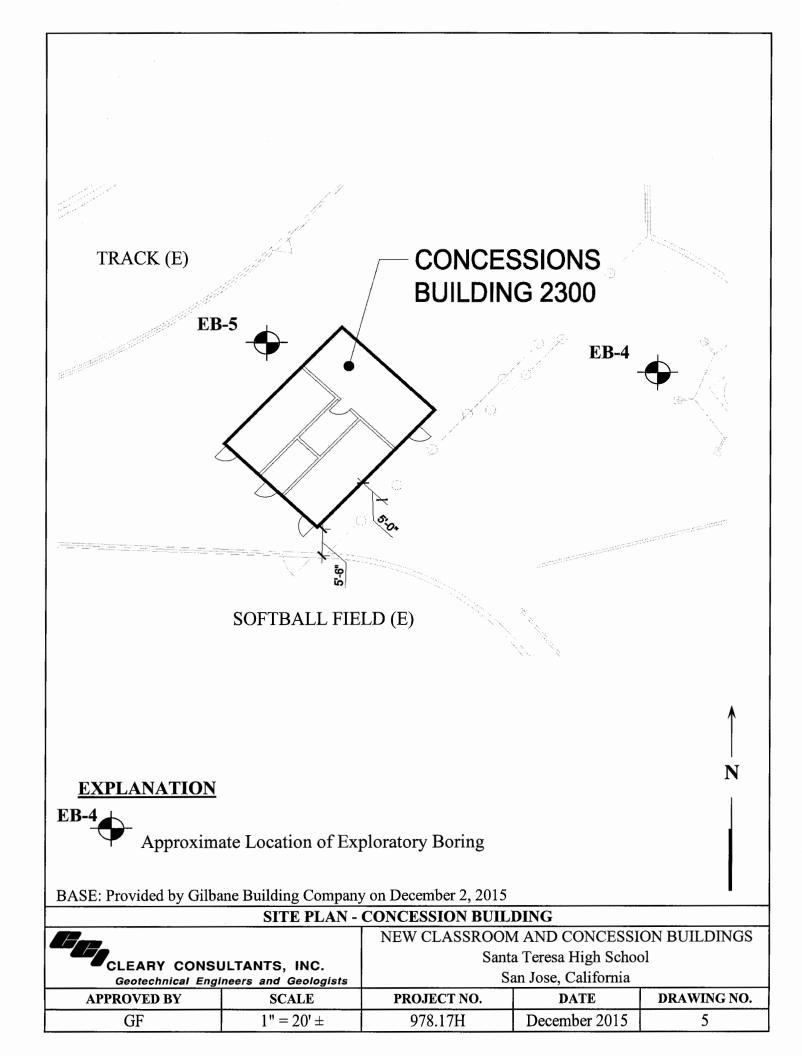


Through 1999; Subsequent Earthquakes through August 2014 plotted in yellow.

REGIONAL EARTHQUAKE EPICENTER MAP NEW CLASSROOM AND CONCESSION BUILDINGS Santa Teresa High School CLEARY CONSULTANTS, INC. San Jose, California Geotechnical Engineers and Geologists APPROVED BY PROJECT NO. DRAWING NO. **SCALE** DATE GF/CC $1" = 25 \text{ miles } \pm$ 978.17H December 2015 3



CLEARY CONSULTANTS, INC. Geotechnical Engineers and Geologists		San	M AND CONCESSI nta Teresa High Schoo San Jose, California	
APPROVED BY	SCALE	PROJECT NO.	DATE	DRAWING NO.
GF	1" = 30' ±	978.17H	December 2015	4



	PRIMARY DIVISION	S	GROUP SYMBOL	SECONDARY DIVISION
	GRAVELS	CLEAN GRAVELS	GW	Well graded gravels, gravel-sand mixtures, little or no fines
LS ERIAL	MORE THAN HALF	(LESS THAN 5% FINES)	GP	Poorly graded gravels or gravel-sand mixtures, little or no fines
COARSE GRAINED SOILS MORE THAN HALF OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	OF COARSE FRACTION IS	GRAVEL WITH	GM	Silty gravels, gravel-sand-silt mixtures, non-plastic fines
GRAINED HALF OF N ER THAN N EVE SIZE	LARGER THAN NO. 4 SIEVE	FINES	GC	Clayey gravels, gravel-sand-clay mixtures, plastic fines
ARSE GRAINE THAN HALF OF LARGER THAN SIEVE SIZE	SANDS	CLEAN SANDS	SW	Well graded sands, gravelly sands, little or no fines
COARSE RE THAN IS LARG	MORE THAN HALF	(LESS THAN 5% FINES)	SP	Poorly graded sands or gravelly sands, little or no fines
CC MORI IS	OF COARSE FRACTION IS	SANDS WITH	SM	Silty sands, sand-silt mixtures, non-plastic fines
	SMALLER THAN NO. 4 SIEVE	FINES	SC	Clayey sands, sand-clay mixtures, plastic fines
ν κ Η	SILTS AND C	LAYS	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity
SOILS LF OF ALLER	LIQUID LIM	IT IS	CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
AINED SC HAN HALF L IS SMAL 200 SIEVE	LESS THAN	50%	OL	Organic silts and organic silty clays of low plasticity
TINE GRAINED SOILS MORE THAN HALF OF TATERIAL IS SMALLER HAN NO. 200 SIEVE SIZI	SILTS AND C	LAYS	МН	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
FINE GRAINED SOILS MORE THAN HALF OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	LIQUID LIM	,	СН	Inorganic clays of high plasticity, fat clays
1	GREATER THA		ОН	Organic clays of medium to high plasticity, organic silts
HI	GHLY ORGANIC SOI	LS	Pt	Peat and other highly organic soils

UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D-2487)

U.S. STANDARD SERIES SIEVE

CLEAR SQUARE SIEVE OPENINGS

20	JU 4	<u>U</u> .	10 4	4 3/	4" 3	··· 1	Z.··
SILTS AND CLAYS		SAND		GRA	AVEL	COBBLES	BOULDERS
SILIS AND CLAIS	FINE	MEDIUM	COARSE	FINE	COARSE	COBBLES	BOULDERS

GRAIN SIZES

SANDS AND GRAVELS	BLOWS/FOOT∳
VERY LOOSE	0 - 4
LOOSE	4 - 10
MEDIUM DENSE	10 - 30
DENSE	30 - 50
VERY DENSE	OVER 50

SILTS AND CLAYS	STRENGTH ★	BLOWS/FOOT
VERY SOFT	0 - 1/4	0 - 2
SOFT	1/4 - 1/2	2 - 4
FIRM	1/2 - 1	4 - 8
STIFF	1 - 2	8 - 16
VERY STIFF	2 - 4	16 - 32
HARD	OVER 4	OVER 32

RELATIVE DENSITY

CONSISTENCY

Number of blows of 140 pound hammer falling 30 inches to drive a 2 inch O.D. (1-3/8 inch I.D.) split barrel (ASTM D-1586).

★ Unconfined compressive strength in tons/sq.ft. as determined by laboratory testing or approximated by the standard penetration test (ASTM D-1586), pocket penetrometer, torvane, or visual observation.

CLEARY CONSULTANTS, INC. Geotechnical Engineers and Geologists

KEY TO EXPLORATORY BORING LOGS

NEW CLASSROOM AND CONCESSION BUILDINGS Santa Teresa High School

San Jose, California

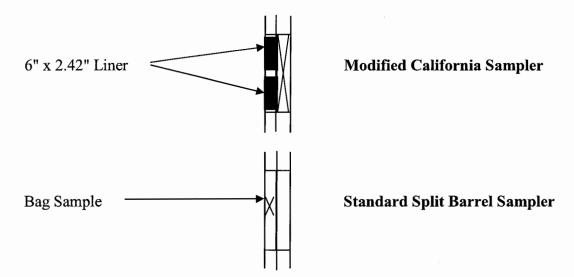
PROJECT NO.	DATE	DRAWING NO.
978.17H	December 2015	6

FIELD SAMPLING PROCEDURES

The soils encountered in the borings were continuously logged in the field by our representative and described in accordance with the Unified Soil Classification System (ASTM D-2487).

Representative soil samples were obtained from the borings at selected depths appropriate to the soil investigation. All samples were returned to our laboratory for classification and testing.

In accordance with the ASTM D1586 procedure, the standard penetration resistance was obtained by dropping a 140 pound hammer through a 30-inch free fall. The 2-inch O.D. Standard split barrel sampler was driven 18 inches or to practical refusal and the number of blows were recorded for each 6-inch penetration interval. The blows per foot recorded on the boring logs represent the accumulated number of blows, or N-value, required to drive the penetration sampler the final 12 inches. In addition, 3.0 inch O.D. x 2.42 inch I.D. drive samples were obtained using a Modified California Sampler and 140 pound hammer. Blow counts for the Modified California Sampler were converted to standard penetration resistance by multiplying by 0.6. The sample type is shown on the boring logs in accordance with the designation below.



Where obtained, the shear strength of the soil samples using either Torvane (TV) or Pocket Penetrometer (PP) devices is shown on the boring logs in the far right hand column.

	SUMMARY OF FIELD SAMPLING PROCEDURES									
	NEW CLASSROOM AND CONCESSION BUILDINGS									
	Santa Teresa High School									
CLEARY CONSULTANTS, INC.	San Jose, California									
Geotechnical Engineers and Geologists	PROJECT NO.	DATE	DRAWING NO.							
·	978.17H	December 2015	7							

LABORATORY TESTING PROCEDURES

The laboratory testing program was directed toward a quantitative and qualitative evaluation of the physical and mechanical properties of the soils underlying the site.

The natural water content was determined on 76 samples of the materials recovered from the borings in accordance with the ASTM D2216 Test Procedure. These water contents are recorded on the boring logs at the appropriate sample depths.

Dry density determinations were performed on 65 samples to measure the unit weight of the subsurface soils in accordance with the ASTM D2937 Test Procedure. The results of these tests are shown on the boring logs at the appropriate sample depths.

Atterberg Limit determinations were performed on eight samples of the subsurface soils in accordance with the ASTM D4318 Test Procedure to determine the ranges of water contents over which the materials exhibited plasticity. The Atterberg Limits are used to classify the soil in accordance with the Unified Soil Classification System and to evaluate the soil's expansion potential. The results of these tests are presented on Drawing 19and on the boring logs at the appropriate sample depths.

The percent soil fraction passing the #4 sieve and #200 sieves was determined on five and 16 samples, respectively, of the subsurface soils in accordance with the ASTM D1140 Test Procedure to aid in the classification of the soils. The results of these tests are shown on the boring logs at the appropriate sample depths.

Free swell tests were performed on 14 samples of the soil materials to evaluate the swelling potential of the soil. The free swell tests were performed by slowly pouring 10 ml of air dried soil passing the No. 40 sieve into a 100 ml graduated cylinder filled with approximately 90 ml of distilled water. The suspension was stirred repeatedly to ensure thorough wetting of the soil specimen. The graduated cylinder was then filled with distilled water to the 100 ml mark and allowed to settle until equilibrium was reached (approximately 24 hours). The free swell volume of the soil was then noted. The percent free swell was calculated by subtracting the initial soil volume from the free swell volume, dividing the difference by the initial volume, and multiplying the result by 100 percent. The results of these tests are presented on the boring logs.

R-Value tests were performed by Cooper Testing Laboratory on representative samples of untreated, and five percent Quicklime Plus (50% Lime/50% Portland cement) treated samples of the subgrade soils to provide data for the pavement design. The tests were performed in accordance with California Test Method 301-F and indicated an R-Value of less than five and 73, respectively, for untreated and chemically treated samples, at an exudation pressure of 300 pounds per square inch. The results of the tests are presented on Drawings 21 and 22.

DRAWING NO. 8

LABORATORY TESTING PROCEDURES CONTINUED

Corrosion testing was performed on a composite sample of the surficial soil materials from the upper five feet of the exploratory borings. Testing included resistivity, pH, chloride and sulfate testing performed in accordance with ASTM G57, ASTM G51, Caltrans 422(modified) and Caltrans 417(modified), respectively. The results of these tests are presented on Drawing 22 and are discussed in Section E. Soil Corrosivity.

EQUIPMENT 8" Dis	ameter Hollow Stem Auger	er* ELEVATION DEPTH TO BEDROCK			LOGGED BY DI Not Enc. DATE DRILLED 10/26				
	Not Enc. PTION AND CLASSIFICA								0/26/2015
DESCRIPTION A		COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR STRENGTH (KSF)
2.5" AC Over 3" AB		Dorle	Medium	SC		PEN RE	ĺO)	DR	ν.
GRAVELLY CLAYEY SAND sand, fine subangular to su to 1/4" diameter	, moist, fine grained brounded gravel up	Dark Yellowish Brown	Dense	SC	- 1 -	12	13	110	PP=3.5
SILTY CLAY, moist, fine grain @1.5': Liquid Limit = 0		Yellowish Brown to Gray	Very Stiff	СН	_ 2 _		21	95	11 – 3.3
@1.5': Liquid Limit = 0 Plasticity Index = Finer than #4 = Finer than #200 = Free Swell = 80					3 -X	25	29	i	
@3.0': Liquid Limit = 0 Plasticity Index = Finer than #200 = Free Swell = 60	59 % = 40 % = 98 % %	Dark Gray	Stiff		- 4	15	30 30	76 90	PP=3.0
					- 6 - \ \ - 7 -	14	28		
@9.5': Finer than #200 =	= 100%	Grayish Brown to Yellowish Brown			9 10 - 11 -	8	29 28	92 93	PP=2.5
		Olive Brown			12 — 13 — 14 — 15 — 16 —	11	29	88 81	
SANDY CLAY, moist, fine gra	ined sand, iron staining	Olive Brown	Stiff	CL	17 —				
@19.5': Liquid Limit = 4 Plasticity Index = Finer than #200 = Free Swell = 10	10 % = 18 % = 72 % %	to Yellowish Brown			18 —	7			
* Drilled with a B56 Truck N PP = Pocket Penetrometer					- 19 - - 20 -	8	29 28	94 90	
THE STRATIFICATION LINES REPRES	ENT THE APPROXIMATE BOUN	DARY BETWE							,
		NITT			ORATORY AND CON				CS
CLEARY CONSUL		NE'	W CLASSK		Teresa High		A DUL	אוועם	O _D
	eers and Geologists		OM NG	Sa	Jose, Cali		DE:	****	NG
APPROVED BY	SCALE	PROJE		n	DATE	_	DRAV		NO.
GF		<u> </u>	.17H	De	cember 201	3		10	

EQUIPMENT 8" Dia	ameter Hollow Stem Auger*	ELEVATI	LOGGED BY DL									
DEPTH TO GROUNDWATER			O BEDRO	CK	Not Enc.				ILLE	D 10		/2015
DESCRI	PTION AND CLASSIFICAT	TION				×	NO E	g F	(%)	ΥTI		F
DESCRIPTION A	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION	(BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR	STRENGTH (KSF)
SANDY CLAY, moist, continu	ed	Olive Brown	Stiff	CL								
		to Vollowish			_ 21 _							
		to Yellowish Brown										
					_ 22 _							
					L ~~							
					_ 23 _							
					L ~ _			:				
					_ 24 _	⅃						
						۱	1	.0	37	85		
					_ 25 _		1	. •	36	81		
					_ 26 _							
					<u> </u>							
					<u> </u>							
		Dark Gray			<u> </u>							
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					— 30 —				21	107		
					-							
					<u> </u>							
					<u> </u>		1					
					— 32 —		İ					
		Olive Gray			⊢ –							
		to Yellowish			— 33 —							
		Brown				1/			22	105		
					─ 34 ─	٧			25	101		
@34.5': Finer than #200 = Free Swell = 20	= 92 %				F -		1	5	23	99		
Tree Swell — 20	70				— 35 —				23	,		
								i				
					- 36 -							
		Ì		ĺ								
		Dark.			- 37 −							
		Dark Grayish Brown			_ 20 _							
					<u> </u>							
					$\begin{bmatrix} \\ \\ \end{bmatrix}$ 39 $\begin{bmatrix} \\ \\ \end{bmatrix}$	\int						
* D.: H. d: 44 D. C. T 1 . 3	Assumed Dis					X.	1	,	27	73		
* Drilled with a B56 Truck N					☐ 40 <u></u>	$/ \setminus$	L		33	90		
THE STRATIFICATION LINES REPRES	ENT THE APPROXIMATE BOUNI	DARY BETWE	EN SOIL TYPE								,	
		NE	W CLASSR								GS	
CLEARY CONSUL				Santa	Teresa Hi	gh	Sch	ool				
	eers and Geologists	PD C T	CIT NO	Sa	n Jose, Ca	lifo	rnia		DB 4 T	LITATO	NT.C	
APPROVED BY GF	SCALE		CT NO. .17H	Do	DATE cember 20	115	+		DRAV	<u>VING</u> 11	NU).
UF		9/8	.1/11	DE	cember 20	113	\perp			ΥY		

EQUIPMENT 8" Dia	meter Hollow Stem Auger*	ELEVATI	ON			LO	GGED	BY		DL	
DEPTH TO GROUNDWATER	Not Enc.	DEPTH T	O BEDRO	CK	Not Enc.)/26/	2015
DESCRIP	TION AND CLASSIFICA	ΓΙΟΝ			DEDOTE	3R	TION 4CE	%) (%)	SITY	~ E	-
DESCRIPTION AN	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR	(KSF)
SANDY CLAY, very moist, con	ntinued	Dark Grayish Brown	Stiff	CL							
					- 42 -						
					 _ 43 _						
@44.5': Finer than #4 = Finer than #200 = Free Swell = 45	85% 73%				44 —		15	22	104		
	%				45	_/\		29	96		:
Bottom of Boring = 45.0'					 _ 46 _						
		i			- 47 -						
:					— — — 48 —						
					- 49 -						
; · · · · · · · · · · · · · · · · · · ·				:	- 50 -						
					51 —			:			
					52 —						
				:	_ 53 _ 						
					— 54 — — —						
×					- 55 						
					- 56 - - -						
				:	- 57 - 						
					- 58 - 						
* Drilled with a B56 Truck N	Mounted Rig	60									
THE STRATIFICATION LINES REPRES	ENT THE APPROXIMATE BOUNI	DARY BETWE	EN SOIL TYP	ES AND	THE TRAN	SITI	ON MAY	BE GRA	DUAL	,	
		NE	LOG OF	OOM	AND CO	NC.	ESSIO			GS	
CLEARY CONSUL Geotechnical Engine					Teresa Hi n Jose, Ca						
APPROVED BY	SCALE	PROJE	CT NO.	54	DATE	TTTO)		DRAV	VING	NO	
GF			.17H	De	cember 20)15		12			

EQUIPMENT 8" Dia	meter Hollow Stem Auger*	ELEVATI	ON			LO	GGED	BY		DL
DEPTH TO GROUNDWATER			O BEDRO	CK	Not Enc.		TE DR) 10	0/26/2015
DESCRIP	TION AND CLASSIFICAT	ΓΙΟΝ				~	TON CE	(%)	ΥT	Ħ
DESCRIPTION AN	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR STRENGTH (KSF)
2.5" AC Over 3" AB CLAYEY SAND, slightly moist subangular to subrounded a diameter	t, fine grained sand, gravel up to 1/4"	Yellowish to Grayish Brown	Loose	SC	_ i _			11	118	
SILTY CLAY, slightly moist, fi	ine grained sand	Brownish Gray	Very Stiff	СН	2 -		7	31	90	PP>4.5
@1.5': Liquid Limit = 5 Plasticity Index = Finer than #200 = Free Swell = 35	= 97% %		:		- 3 - 	X 17	16	27		
					- 4 - - 5 -		32	27 21	90 94	PP>4.5
					_	X	19	26		
SANDY TO SILTY CLAY, mo	ist	Grayish	Stiff	CL	- 7 - - 8 -					
@9.5': Liquid Limit = 4 Plasticity Index = Finer than #200 = Free Swell = 35	.9 % - 24 %	Brown			_ 9 <u>_</u>		8	32 31	87 85	PP=1.75
Finer than #200 = Free Swell = 35	= 99% %				— 10 — — — — — 11 —					
		Yellowish to Grayish Brown			— 12 — — 13 — — 14 —					
					 _ 15 _ 		8	33	88	
					— 16 — — — — — 17 —					
* D. W. 1. 12 . D. C. T. 1. 1.	A I D'				- 18 - 19 -					
* Drilled with a B56 Truck N PP = Pocket Penetrometer Bottom of Boring = 20.0'	nounted kig	·			_ 19 _ 	Å	14	31 31	91 89	
THE STRATIFICATION LINES REPRES	ENT THE APPROXIMATE BOUNI	DARY BETWE								
		NEV	LOG OF I							GS
CLEARY CONSUL		, AL	CLIBOR	Santa	Teresa Hi	gh	School	. 201		
Geotechnical Engine		DD A Y	OT NO	Saı	n Jose, Ca	lifo		DD A F	UTNIC	NO
APPROVED BY	SCALE	PROJE	CT NO.		DATE			DRAV	VING	NU.

978.17H

December 2015

13

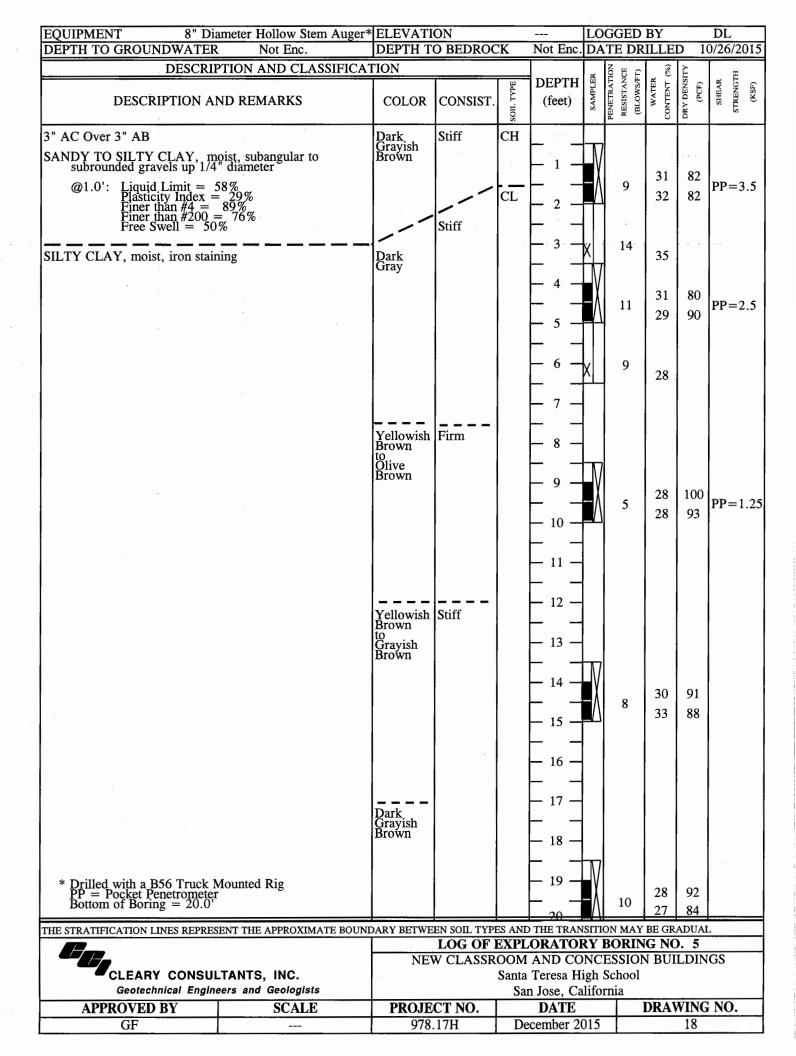
EQUIPMENT 8" Dia	ameter Hollow Stem Auger*						GGED			DL
DEPTH TO GROUNDWATER	Not Enc.	DEPTH T	O BEDROC	CK	Not Enc.		TE DR) 10	0/26/2015
DESCRI	PTION AND CLASSIFICAT	TION			DESCRIPTION	K.	TON ICE	(%)	н	H
DESCRIPTION A	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR STRENGTH (KSF)
2.5" AC Over 2" AB		Dark Grayish Brown	Stiff	СН		_				
SILTY CLAY, moist, fine grain subangular gravel	ned sand, fine	Brown			- 1 -	-\		11	114	
@1.5': Liquid Limit = 0 Plasticity Index = Finer than #200 = Free Swell = 55	53 % = 34 % = 97 % %				_ 2 _		12	33	89	PP>4.5
@3.0': very moist					3 -	X,	16	32		
@4.5': increased silt cont	tent				_ 4 <u>_</u>		11	33	71 87	
					_ 5 _ 			33	67	
					<u> </u>	X	10	31		
Bottom of Boring = 6.5			:		7 -					
					<u> </u>					
				į	8 -					
					_ 9 _					
					— 10 —					
					— 11 —					
			·		<u> </u>					
					<u> </u>					
					├ .					
					— 14 —					
				,	— 15 —					
					16					
					— 17 —					
					- 18 -					
					10					
* Drilled with a P56 Truck N	Mounted Rig				- 19 -					
* Drilled with a B56 Truck N PP = Pocket Penetrometer										
THE STRATIFICATION LINES REPRES	SENT THE APPROXIMATE BOUND	LOG OF EXPLORATORY BORING NO. 3								
CLEARY CONSUL	TANTS INC	NE	W CLASSR	OOM	AND CO	NC	ESSION			GS
Geotechnical Engine	eers and Geologists				Teresa Hi n Jose, Ca		rnia			
APPROVED BY GF	SCALE		CT NO.	Da	DATE ecember 20	715		DRAV	VING 14	NO.
Ur Ur		910	. 1 / 11	DE	compet 20	113			14	

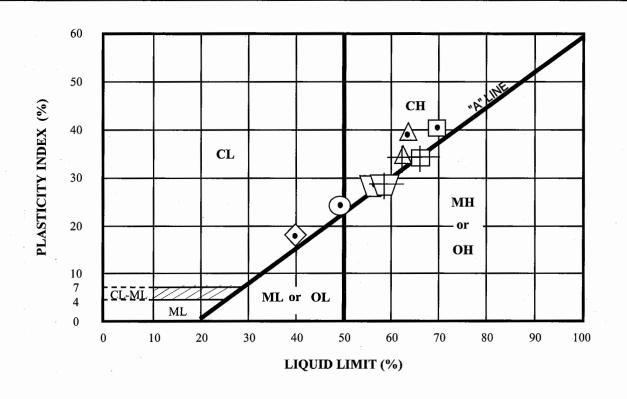
EQUIPMENT 8" Dia	meter Hollow Stem Auger*			-		LO	GGED	BY		DL
DEPTH TO GROUNDWATER	37.0' ±	DEPTH T	O BEDRO	CK	Not Enc.	DA	TE DR	ILLEI) 10	0/26/2015
DESCRIP	TION AND CLASSIFICAT	ΓΙΟΝ			DESC	E.	TION (CE	(%)	YTT	E
DESCRIPTION AN	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR STRENGTH (KSF)
2.5" AC Over 3" AB SILTY CLAY, moist, trace fine subangular to subrounded gameter @1.5': Liquid Limit = 6 Plasticity Index = Finer than #200 = Free Swell = 60°		Dark Grayish Brown	Stiff	CH-	_ 1 _ _ 1 _ _ 2 _		8	35 41	90 73	PP=3.0
		Grayish Brown			3 — 4 — — 5 — — 6 — —	× ×	13	26 27 30 27	89	PP>4.5
@9.5': Finer than #200 = Free Swell = 0%		Dark Gray	Firm		- 7 - - 8 - - 9 - - 10 - - 11 -		5	26 30	94 91	PP=1.75
@14.5': very moist					- 12 13 14 15 16 17 17 17 17		5	31 34	90 84	
@19.5': Finer than #200 = Free Swell = 200 * Drilled with a B56 Truck MPP = Pocket Penetrometer	= 99% %	Dark Yellowish Brown to Olive Brown	Stiff	CL	18 18 19		10	18 27	88 94	
					20	/ \		32	80	
THE STRATIFICATION LINES REPRES	TANTS, INC.		LOG OF W CLASSR	EXPL OOM Santa	ORATOI AND CO Teresa H	NC igh	BORIN ESSION School	G NO	. 4	
Geotechnical Engine				Sa	n Jose, Ca	lifoı				
APPROVED BY GF	SCALE 		CT NO. .17H	De	DATE cember 20	015		DRAV	VING 15	NO.

EQUIPMENT 8" Dia	meter Hollow Stem Auger	*ELEVATI	ON	-		LO	GGED	BY		DL	П
DEPTH TO GROUNDWATER	37.0' ±	DEPTH T	O BEDRO	CK	Not Enc.		TE DR) 10	0/26/20	15
DESCRIP	TION AND CLASSIFICA	TION			DEDES	K	TION (CE	(%)	SITY	T H	
DESCRIPTION AN	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT	DRY DENSITY (PCF)	SHEAR	(KSF)
SILTY CLAY, moist, continued	····	Dark Yellowish Brown	Stiff	CL							
		to Olive Brown			 _ 22 _						
			1		 - 23 -						
					 _ 24 _			26	96		
					 _ 25 _		10	31 40	90 80		į
				:	 _ 26 _						
					- 27 -						
		Dark Gray			_ 28						
©20 51. Einen then #200 —	. 07.0		:		29 —		14	28	95		
@29.5': Finer than #200 = Free Swell = 405	6 91 76 76				_ 30 _	_/\	14	28	90		
					<u> </u>						
GRAVELLY SILTY SAND, mo subangular to subrounded g diameter, iron staining	oist, fine grained sand, ravel up to 3/4"	Dark Grayish Brown	Dense	SP- SM	- 32 - - 33 -						
@34.5': Finer than #4 = Finer than #200 =	59% 4%			SP	_ 34 _ _ 34 _		40	8 5 5	117 116 116		
					- 35 - - 36 -						
					 _ 37 _			¥			
* Drilled with a B56 Truck M				38			록				
Water level as encountered Water level as measured aft PP = Pocket Penetrometer				39 —	\bigvee	25					
					L 40 _			DE 65	A D		\dashv
THE STRATIFICATION LINES REPRESE	ENT THE APPROXIMATE BOUN	DARY BETWE	LOG OF								\dashv
CLEARY CONSUL Geotechnical Engine		NEV	W CLASSR	OOM Santa	AND CO Teresa H	NC igh	ESSION School			GS	
APPROVED BY	SCALE	PROJE	CT NO.	Sa	n Jose, Ca	11110		DRAV	VING	NO.	\dashv
GF				De	PROJECT NO. DATE DRAWING 978.17H December 2015 16						٦

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	EQUIPMENT 8" Dia	ameter Hollow Stem Auger*	EQUIPMENT 8" Diameter Hollow Stem Auger* ELEVATION LOGGED BY DL										
	DEPTH TO GROUNDWATER	37.0' ±	DEPTH T	O BEDROC	CK	Not Enc.				D 10)/26/	2015	
	DESCRI	PTION AND CLASSIFICAT	TION			DEPOS	J.R	TON (CE	(%)	тту		Ξ	
	DESCRIPTION A	ND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (feet)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	SHEAR	STRENGTH (KSF)	
	GRAVELLY SILTY SAND, m	oist, continued	Dark Grayish Brown	Dense	SP- SM	 _ 41 _							
:	: - <u> </u>				- 42 -								
:	SANDY CLAY, very moist, fir	ne grained sand	Olive Brown	Very Stiff	CL	 _ 43 _	.:						
	@44.0': Finer than #4 = Finer than #200 = Free Swell = 5%	100% = 62%				— 44 — — 45		23	25 28	104 99			
	Bottom of Boring = 45.0'				-	47		-			-		
						46							
		•				<u> </u>							
			:			- 48 - 49							
	:					_ 49 _ 50 _							
			Þ	:		 _ 51 _							
						_ 52 _							
						_ 53 _ 							
						— 54 — — —							
						55 56							
						_							
						— — — 58 —							
	* Drilled with a B56 Truck N PP = Pocket Penetrometer	Mounted Rig	59 —										
	THE STRATIFICATION LINES REPRES	SENT THE APPROXIMATE BOUNT	ATE BOUNDARY BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL						,				
			LOG OF EXPLORATORY BORING NO. 4 NEW CLASSROOM AND CONCESSION BUILDINGS										
	CLEARY CONSUL	LTANTS, INC.	NEV	w CLASSR		AND CO Teresa H				LDIN	GS		
	Geotechnical Engine	eers and Geologists				n Jose, Ca		rnia					
	APPROVED BY	SCALE	PROJE			DATE	115		DRAV		NO		
	GF		978.17H December 2015 17										





KEY SYMBOL	BORING NO.	SAMPLE DEPTH (feet)	NATURAL WATER CONTENT %	LIQUID LIMIT %	PLASTICITY INDEX %	PASSING NO. 200 SIEVE %	LIQUIDITY	UNIFIED SOIL CLASSIFICATION SYMBOL
À	1	1.5	21	64	38	55	-0.1	СН
•	1	3.0	29	69	40	98	0.0	СН
⋄	1	19.5	28	40	18	72	0.3	CL
•	2	1.5	31	58	27	97	0.0	СН
\odot	2	9.5	31	49	24	99	0.3	CL
4	3	1.5	33	63	34	97	0.1	СН
#	4	1.5	41	66	34	92	0.3	СН
\Leftrightarrow	5	1.0	31	58	29	76	0.1	СН

CLEARY CONSULTANTS, INC. Geotechnical Engineers and Geologists

PLASTICITY CHART NEW CLASSROOM AND CONCESSION BUILDING Santa Teresa High School

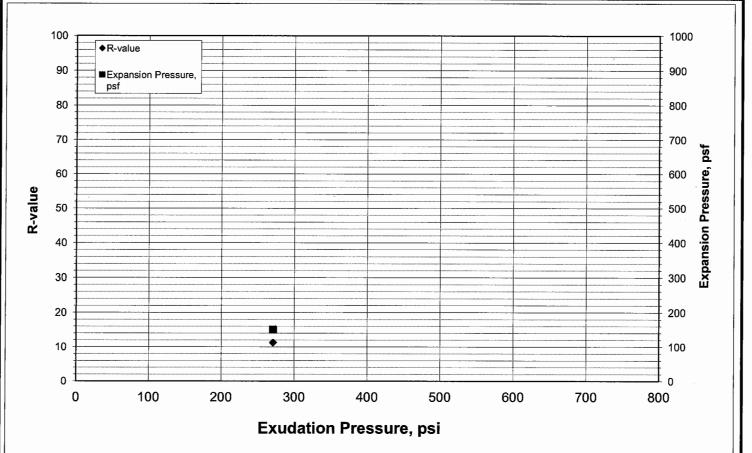
Santa Teresa High School San Jose, California

PROJECT NO.	DATE	DRAWING NO.
978.17H	December 2015	19



R-value Test Report (Caltrans 301)

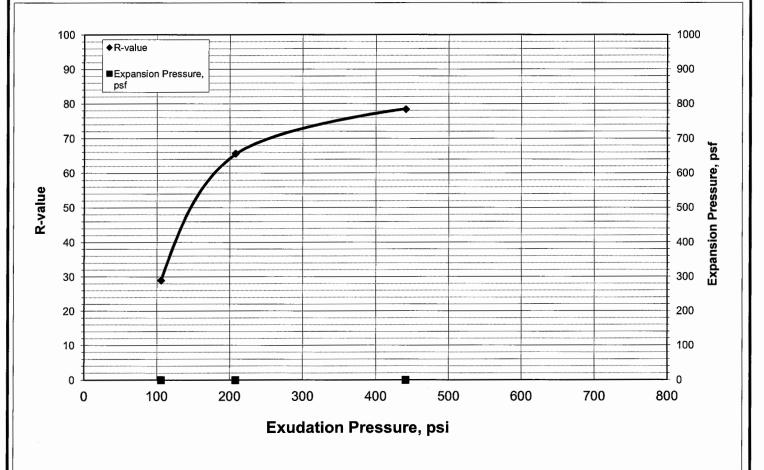
	quadratic tempor, per 5 (no. 1), 5 Paris major (1) Paris Cont. Subject State (1) and Electric and the second secon	and the state of t	and New cresci offstale subseque 24	Service of an indicated address and and and a fine	زوولييارمارتيا طلق بالتعلمانات للكمتا ماعصساء	The case for the Control of the cont	resolution who are sufficiently the Commerce which all their because are below the behind the behind the property of
Job No.:	018-829			Date:	11/13/15	Initial Moisture,	15.3%
Client:	Cleary Consultants			Tested	MD	R-value by	
Project:	New Classroom Buildin	g Poenix HS -	978.17H	Reduced	RU	Stabilometer	<5
Sample	EB 1,3 @ 0.5-5.0'			Checked	DC	Expansion	
Soil Type:	Type: Very Dark Grayish Brown CLAY w/ Sand			-		Pressure	psf
Spe	ecimen Number	Α	В	С	D	Rem	arks:
Exudation	Pressure, psi	271				Soil extruded from the	mold giving a false
Prepared	Weight, grams	1200				exudation pressure. P	er Caltrans, the R-
Final Wate	er Added, grams/cc	82				Value test was termina	ated and an R-Value of
Weight of	Soil & Mold, grams	3039				less then 5 was report	ed.
Weight of	Mold, grams	2102				1	
Height Aft	ter Compaction, in.	2.32]	
Moisture	Content, %	23.2				1	
Dry Densi	ty, pcf	99.3				1	
Expansion	n Pressure, psf	150.5				Ī	
Stabilome	eter @ 1000					1	
Stabilome	eter @ 2000	136				1	
Turns Dis	placement	3.23					
R-value		11					





R-value Test Report (Caltrans 301)

Market of Washington		North Habitet Co. N. S. Alexandre	Mark Control of the Control		E BATA NAMA SEMBANGANI		elemente l'ordants i des sir	
Job No.:	018-829			Date:	11/13/15	Initial Moisture,	25.6%	<u>)</u>
Client:	Cleary Consultants			Tested	MD	R-value by	73	
Project:	New Classroom Building	at Phoenix H	S - 978.17H	Reduced	RU	Stabilometer	13	
Sample	EB 1,3 @ 0.5-5.0	EB 1,3 @ 0.5-5.0		Checked	DC	Expansion		nof
Soil Type:	Very Dark Grayish Brown CLA	Y w/ Sand (+2.5	% Portland Ce	ment + 2.5% H	iCal Quicklime)	Pressure	0	psf
Spe	cimen Number	Α	В	С	D	Rem	arks:	
Exudation	Pressure, psi	106	208	441				
Prepared \	Weight, grams	1200	1200	1200]		
Final Wate	er Added, grams/cc	553	0	-20				
Weight of	Soil & Mold, grams	19	2971	3041				
Weight of	Mold, grams	2077	2098	2077				
Height Aft	er Compaction, in.	2.5	2.3	2.46				
Moisture (Content, %	83.5	25.6	23.5	- 1			
Dry Densi	ty, pcf	-135.9	91.5	96.1	-			
Expansion	n Pressure, psf	0.0	0.0	0.0				
Stabilome	ter @ 1000							
Stabilome	ter @ 2000	114	50	32				
Turns Dis	placement	2.48	2.4	2.68				
R-value		29	66	78				





Corrosivity Tests Summary

CTL#	018-	829		Date:		01		Tested By:	PJ		Checked:		PJ	-
Client: Remarks:	Clea	ary Consulta	nts	Project:	N	ew Classroor	n Building a	t Phoenix F	15		Proj. No:	978	3.17H	
	ple Location	or ID	Resistiv	Resistivity @ 15.5 °C (Ohm-cr		Chloride	Sul	fate	рН	OR	P	Sulfide	Moisture	
			As Rec.	Min	Sat.	mg/kg	mg/kg	%		(Red		Qualitative	At Test	
						Dry Wt.	Dry Wt.	Dry Wt.		E _H (mv)	At Test	by Lead	%	Soil Visual Description
Boring	Sample, No.	Depth, ft.	ASTM G57	Cal 643	ASTM G57	ASTM D4327	ASTM D4327	ASTM D4327	ASTM G51	ASTM G200	Temp °C	Acetate Paper	ASTM D2216	
EB 1,3	-	0.5-5.0	-	-	1,820	7	217	0.0217	8.0	533	18	-	15.5	Olive Gray Sandy CLAY

APPENDIX A

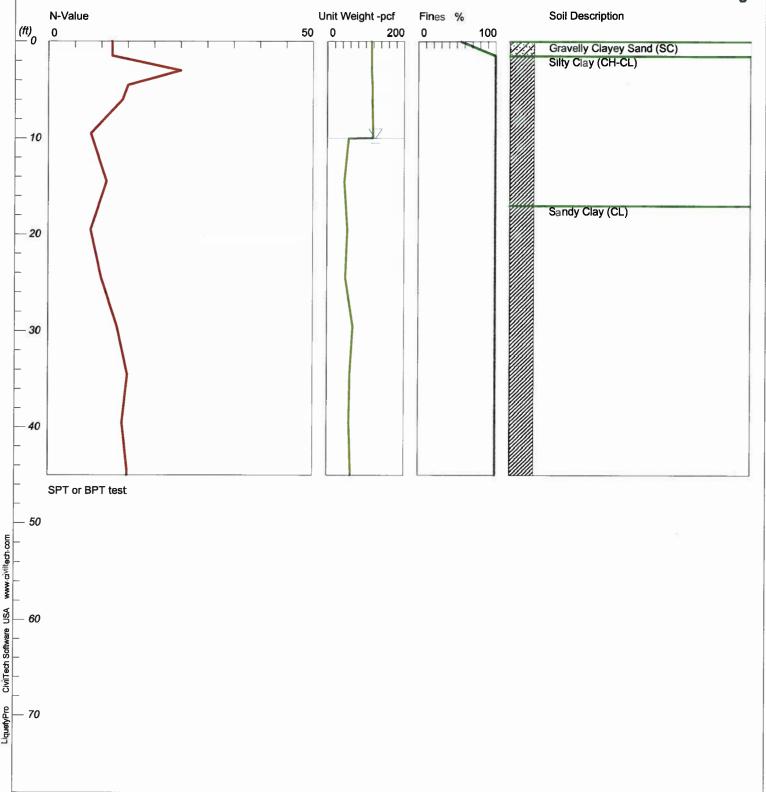
Santa Teresa High School New Classroom and Concession Building, Liquefaction and Dry Settlement Analyses and Calculations, EB-1 and EB-4, Drilled October 26, 2015

LIQUEFACTION ANALYSIS

Santa Teresa HS Classroom and Concession Buildings

Hole No.=EB-1 Water Depth=10 ft

Magnitude=8.5 Acceleration=0.735g

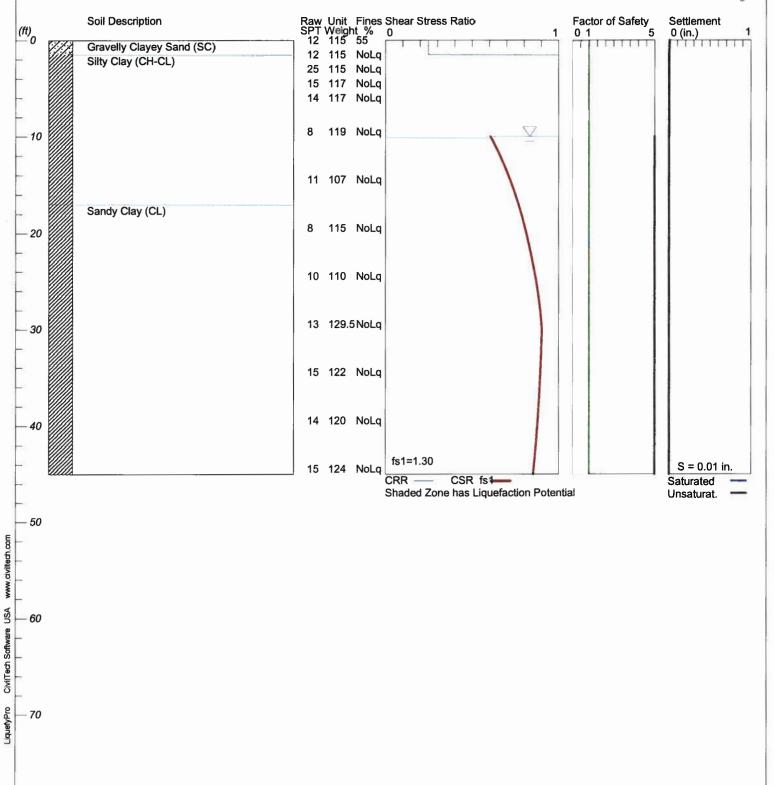


LIQUEFACTION ANALYSIS

Santa Teresa HS Classroom and Concession Buildings

Hole No.=EB-1 Water Depth=10 ft

Magnitude=8.5 Acceleration=0.735g



Santa Teresa HS Class Concession EB1.sum

LIQUEFACTION ANALYSIS SUMMARY

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Ce = 1.25

********************************* *******

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Input File Name: C:\Grant Roughs\Liquefy Pro Data Files\Santa Teresa HS Class Concession EB1.liq

Title: Santa Teresa HS Classroom and Concession Buildings Subtitle:

Surface Elev.= Hole No.=EB-1
Depth of Hole= 45.00 ft
Water Table during Earthquake= 10.00 ft
Water Table during In-Situ Testing= 10.00 ft
Max. Acceleration= 0.74 g
Earthquake Magnitude= 8.50

Input Data:

Surface Elev.= Hole No.=EB-1 Depth of Hole=45.00 ft Water Table during Earthquake= 10.00 ft Water Table during In-Situ Testing= 10.00 ft Max. Acceleration=0.74 g Earthquake Magnitude=8.50 No-Liquefiable Soils: CL, OL are Non-Liq. Soil

- 1. SPT or BPT Calculation.
- 2. Settlement Analysis Method: Tokimatsu, M-correction
- 3. Fines Correction for Liquefaction: Idriss/Seed
- Fine Correction for Settlement: During Liquefaction*
 Settlement Calculation in: All zones*
- 6. Hammer Energy Ratio,
- 7. Borehole Diameter,

Cb=18. Sampling Method, Cs=1

9. User request factor of safety (apply to CSR), User= 1.3 Plot one CSR curve (fs1=User)

10. Use Curve Smoothing: Yes* * Recommended Options

In-Situ Depth ft	Test Dar SPT	ta: gamma pcf	Fines %
0.00	12.00	115.00	55.00
1.50	12.00	115.00	NoLiq
3.00	25.00	115.00	NoLiq
4.50	15.00	117.00	NoLiq
6.00	14.00	117.00	NoLiq
9.50	8.00	119.00	NoLiq
14.50	11.00	107.00	NoLiq
19.50	8.00	115.00	NoLiq
24.50	10.00	110.00	NoLiq

Santa Teresa HS Class Concession EB1.sum

29.50	13.00	129.50	NoLiq
34.50	15.00	122.00	NoLiq
39.50	14.00	120.00	NoLiq
44.50	15.00	124.00	NoLiq

Output Results:
Settlement of Saturated Sands=0.00 in.
Settlement of Unsaturated Sands=0.01 in.
Total Settlement of Saturated and Unsaturated Sands=0.01 in.
Differential Settlement=0.003 to 0.004 in.

Depth ft	CRRM	CSRfs	F.S.	S_sat. in.	S_dry in.	S_all
	0.25 0.25 0.25 0.25 0.25 0.25 0.25 0.25	0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62	5.00 5.00			
2.25 2.30 2.35	2.00 2.00 2.00	0.62 0.62 0.62	5.00 5.00 5.00	0.00 0.00 0.00 Page 2	0.00 0.00 0.00	0.00 0.00 0.00

		Conto To	noco 110	Class Co	ncoccion	ED1 cum
2.40	2.00	0.62	5.00	0.00	0.00	EB1.sum 0.00
2.45	2.00	0.62	5.00	0.00	0.00	0.00
2.50	2.00	0.62	5.00	0.00	0.00	0.00
2.55	2.00	0.62	5.00	0.00	0.00	0.00
2.60	2.00	0.62	5.00	0.00	0.00	0.00
2.65	2.00	0.62	5.00	0.00	0.00	0.00
2.70	2.00	0.62	5.00	0.00	0.00	0.00
2.75	2.00	0.62	5.00	0.00	0.00	0.00
2.80	2.00	0.62	5.00	0.00	0.00	0.00
2.85	2.00	0.62	5.00	0.00	0.00	0.00
2.90	2.00	0.62	5.00	0.00	0.00	0.00
2.95	2.00	0.62	5.00	0.00	0.00	0.00
3.00 3.05	2.00	0.62 0.62	5.00 5.00	0.00 0.00	0.00	0.00 0.00
3.10	2.00	0.62	5.00	0.00	0.00 0.00	0.00
3.15	2.00	0.62	5.00	0.00	0.00	0.00
3.20	2.00	0.62	5.00	0.00	0.00	0.00
3.25	2.00	0.62	5.00	0.00	0.00	0.00
3.30	2.00	0.62	5.00	0.00	0.00	0.00
3.35	2.00	0.62	5.00	0.00	0.00	0.00
3.40	2.00	0.62	5.00	0.00	0.00	0.00
3.45	2.00	0.62	5.00	0.00	0.00	0.00
3.50	2.00	0.62	5.00	0.00	0.00	0.00
3.55	2.00	0.62	5.00	0.00	0.00	0.00
3.60	2.00	0.62	5.00	0.00	0.00	0.00
3.65	2.00	0.62 0.62	5.00 5.00	0.00	0.00	0.00
3.70 3.75	2.00 2.00	0.62	5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
3.80	2.00	0.62	5.00	0.00	0.00	0.00
3.85	2.00	0.62	5.00	0.00	0.00	0.00
3.90	2.00	0.62	5.00	0.00	0.00	0.00
3.95	2.00	0.62	5.00	0.00	0.00	0.00
4.00	2.00	0.62	5.00	0.00	0.00	0.00
4.05	2.00	0.62	5.00	0.00	0.00	0.00
4.10	2.00	0.62	5.00	0.00	0.00	0.00
4.15 4.20	2.00 2.00	0.62	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
4.25	2.00	0.61 0.61	5.00	0.00	0.00	0.00
4.30	2.00	0.61	5.00	0.00	0.00	0.00
4.35	2.00	0.61	5.00	0.00	0.00	0.00
4.40	2.00	0.61	5.00	0.00	0.00	0.00
4.45	2.00	0.61	5.00	0.00	0.00	0.00
4.50	2.00	0.61	5.00	0.00	0.00	0.00
4.55	2.00	0.61	5.00	0.00	0.00	0.00
4.60	2.00	0.61	5.00	0.00	0.00	0.00
4.65	2.00	0.61	5.00	0.00	0.00	0.00
4.70	2.00	0.61	5.00	0.00	$0.00 \\ 0.00$	0.00
4.75 4.80	2.00 2.00	0.61 0.61	5.00 5.00	0.00 0.00	0.00	0.00 0.00
4.85	2.00	0.61	5.00	0.00	0.00	0.00
4.90	2.00	0.61	5.00	0.00	0.00	0.00
4.95	2.00	0.61	5.00	0.00	0.00	0.00
5.00	2.00	0.61	5.00	0.00	0.00	0.00
5.05	2.00	0.61	5.00	0.00	0.00	0.00
5.10	2.00	0.61	5.00	0.00	0.00	0.00
5.15	2.00	0.61	5.00	0.00	0.00	0.00
5.20 5.25	2.00 2.00	0.61 0.61	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
5.30	2.00	0.61	5.00	0.00	0.00	0.00
5.35	2.00	0.61	5.00	0.00	0.00	0.00
5.40	2.00	0.61	5.00	0.00	0.00	0.00
5.45	2.00	0.61	5.00	0.00	0.00	0.00
5.50	2.00	0.61	5.00	0.00_	0.00	0.00
				Dago 3		

		Santa Te	resa HS	class Cor	ncession	EB1.sum
5.55	2.00	0.61	5.00	0.00	0.00	0.00
5.60 5.65	2.00	0.61 0.61	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$
5.70	2.00	0.61	5.00	0.00	0.00	0.00
5.75	2.00	0.61	5.00	0.00	0.00	0.00
5.80	2.00	0.61	5.00	0.00	0.00	0.00
5.85 5.90	2.00 2.00	$\substack{0.61\\0.61}$	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
5.95	2.00	0.61	5.00	0.00	0.00	0.00
6.00	2.00	0.61	5.00	0.00	0.00	0.00
6.05 6.10	2.00 2.00	$0.61 \\ 0.61$	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
6.15	2.00	0.61	5.00	0.00	0.00	0.00
6.20	2.00	0.61	5.00	0.00	0.00	0.00
6.25 6.30	2.00 2.00	$\substack{0.61\\0.61}$	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
6.35	2.00	0.61	5.00	0.00	0.00	0.00
6.40	2.00	0.61	5.00	0.00	0.00	0.00
6.45 6.50	2.00	$\substack{0.61\\0.61}$	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
6.55	2.00 2.00	0.61	5.00	0.00	0.00	0.00
6.60	2.00	0.61	5.00	0.00	0.00	0.00
6.65	2.00	0.61	5.00	0.00	0.00	0.00
6.70 6.75	2.00 2.00	0.61 0.61	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
6.80	2.00	0.61	5.00	0.00	0.00	0.00
6.85	2.00	0.61	5.00	0.00	0.00	0.00
6.90 6.95	2.00 2.00	$\substack{0.61\\0.61}$	5.00 5.00	0.00 0.00	0.00 0.00	0.00
7.00	2.00	0.61	5.00	0.00	0.00	0.00
7.05	2.00	0.61	5.00	0.00	0.00	0.00
7.10 7.15	2.00 2.00	0.61 0.61	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
7.20	2.00	0.61	5.00	0.00	0.00	0.00
7.25	2.00	0.61	5.00	0.00	0.00	0.00
7.30 7.35	2.00 2.00	$\substack{0.61\\0.61}$	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
7.40	2.00	0.61	5.00	0.00	0.00	0.00
7.45	2.00	0.61	5.00	0.00	0.00	0.00
7.50 7.55	2.00	$0.61 \\ 0.61$	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
7.60	2.00	0.61	5.00	0.00	0.00	0.00
7.65	2.00	0.61	5.00	0.00	0.00	0.00
7.70 7.75	2.00 2.00	$\substack{0.61\\0.61}$	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
7.80	2.00	0.61	5.00	0.00	0.00	0.00
7.85	2.00	0.61	5.00	0.00	0.00	0.00
7.90 7.95	2.00 2.00	$\substack{0.61\\0.61}$	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
8.00	2.00	0.61	5.00	0.00	0.00	0.00
8.05	2.00	0.61	5.00	0.00	0.00	0.00
8.10 8.15	2.00	$\substack{0.61\\0.61}$	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
8.20	2.00	0.61	5.00	0.00	0.00	0.00
8.25	2.00	0.61	5.00	0.00	0.00	0.00
8.30 8.35	2.00 2.00	$\begin{array}{c} 0.61 \\ 0.61 \end{array}$	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
8.40	2.00	0.61	5.00	0.00	0.00	0.00
8.45	2.00	0.61	5.00	0.00	0.00	0.00
8.50 8.55	2.00	$\substack{0.61\\0.61}$	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
8.60	2.00	0.61	5.00	0.00	0.00	0.00
8.65	2.00	0.61	5.00	0.00	0.00	0.00
				Page 4		

Santa Teresa HS Class Concession EB1. sum					6] 6		ED1
8.75	9 70	2 00					
8.80 2.00 0.61 5.00 0.00 0.00 0.00 8.85 2.00 0.61 5.00 0.00 0.00 0.00 8.95 2.00 0.61 5.00 0.00 0.00 0.00 9.00 2.00 0.61 5.00 0.00 0.00 0.00 9.05 2.00 0.61 5.00 0.00 0.00 0.00 9.10 2.00 0.61 5.00 0.00 0.00 0.00 9.15 2.00 0.61 5.00 0.00 0.00 0.00 9.20 2.00 0.61 5.00 0.00 0.00 0.00 9.35 2.00 0.61 5.00 0.00 0.00 0.00 9.35 2.00 0.61 5.00 0.00 0.00 0.00 9.45 2.00 0.61 5.00 0.00 0.00 0.00 9.50 2.00 0.61 5.00 0.00 0.00				5.00			
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	10.85	2.00			0.00	0.00	
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11.05	2.00					
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$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11.13	2.00					
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11.20		0.64				0.00
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	11.25	2.00	0.64				
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11.80 2.00 0.66 5.00 0.00 0.00 0.00	11.75	2.00	0.66	5.00	0.00	0.00	0.00
	11.80	2.00	0.66	5.00		0.00	0.00

		C T-		67 6	-	-p1
11.85	2.00	0.66	5.00	Class Cor	0.00	EB1.sum 0.00
11.90	2.00	0.66	5.00	0.00	0.00	0.00
11.95	2.00	0.66	5.00	0.00	0.00	0.00
12.00	2.00	0.66	5.00	0.00	0.00	0.00
12.05	2.00	0.66	5.00	0.00	0.00	0.00
12.10	2.00	0.67	5.00	0.00	0.00	0.00
12.15	2.00	0.67	5.00	0.00	0.00	0.00
12.20	2.00	0.67	5.00	0.00	0.00	0.00
12.25 12.30	2.00 2.00	0.67 0.67	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00
12.35	2.00	0.67	5.00	0.00	0.00	0.00
12.40	2.00	0.67	5.00	0.00	0.00	0.00
12.45	2.00	0.67	5.00	0.00	0.00	0.00
12.50	2.00	0.68	5.00	0.00	0.00	0.00
12.55	2.00	0.68	5.00	0.00	0.00	0.00
12.60	2.00	0.68	5.00	0.00	0.00	0.00
12.65 12.70	2.00 2.00	0.68 0.68	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
12.75	2.00	0.68	5.00	0.00	0.00	0.00
12.80	2.00	0.68	5.00	0.00	0.00	0.00
12.85	2.00	0.68	5.00	0.00	0.00	0.00
12.90	2.00	0.69	5.00	0.00	0.00	0.00
12.95 13.00	2.00	0.69 0.69	5.00 5.00	0.00 0.00	0.00	0.00
13.05	2.00	0.69	5.00	0.00	0.00 0.00	$0.00 \\ 0.00$
13.10	2.00	0.69	5.00	0.00	0.00	0.00
13.15	2.00	0.69	5.00	0.00	0.00	0.00
13.20	2.00	0.69	5.00	0.00	0.00	0.00
13.25	2.00	0.69	5.00	0.00	0.00	0.00
$13.30 \\ 13.35$	2.00 2.00	0.69 0.70	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
13.40	2.00	0.70	5.00	0.00	0.00	0.00
13.45	2.00	0.70	5.00	0.00	0.00	0.00
13.50	2.00	0.70	5.00	0.00	0.00	0.00
13.55	2.00	0.70	5.00	0.00	0.00	0.00
13.60	2.00	0.70	5.00	0.00	0.00	0.00
13.65 13.70	2.00	0.70 0.70	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
13.75	2.00	0.70	5.00	0.00	0.00	0.00
13.80	2.00	0.71	5.00	0.00	0.00	0.00
13.85	2.00	0.71	5.00	0.00	0.00	0.00
13.90	2.00	0.71	5.00	0.00	0.00	0.00
13.95 14.00	2.00	0.71 0.71	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
14.05	2.00	0.71	5.00	0.00	0.00	0.00
14.10	2.00	0.71	5.00	0.00	0.00	0.00
14.15	2.00	0.71	5.00	0.00	0.00	0.00
14.20	2.00	0.71	5.00	0.00	0.00	0.00
14.25	2.00	0.72	5.00	0.00	0.00	0.00
14.30 14.35	2.00	0.72 0.72	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
14.40	2.00	0.72	5.00	0.00	0.00	0.00
14.45	2.00	0.72	5.00	0.00	0.00	0.00
14.50	2.00	0.72	5.00	0.00	0.00	0.00
14.55	2.00	0.72	5.00	0.00	0.00	0.00
14.60 14.65	2.00	0.72 0.72	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
14.70	2.00	0.72	5.00	0.00	0.00	0.00
14.75	2.00	0.73	5.00	0.00	0.00	0.00
14.80	2.00	0.73	5.00	0.00	0.00	0.00
14.85	2.00	0.73	5.00	0.00	0.00	0.00
14.90	2.00	0.73 0.73	5.00 5.00	0.00	0.00	0.00 0.00
14.95	2.00	0.73	3.00	0.00	0.00	0.00

		Santa Te	resa HS	class cor	ncession	EB1.sum
15.00	2.00	0.73	5.00	0.00	0.00	0.00
15.05 15.10	2.00	0.73 0.73	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$
15.15	2.00	0.73	5.00	0.00	0.00	0.00
15.20	2.00	0.74	5.00	0.00	0.00	0.00
15.25	2.00	0.74	5.00	0.00	0.00	0.00
15.30 15.35	2.00	0.74 0.74	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
15.40	2.00	0.74	5.00	0.00	0.00	0.00
15.45	2.00	0.74	5.00	0.00	0.00	0.00
15.50 15.55	2.00	0.74 0.74	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00
15.60	2.00	0.74	5.00	0.00	0.00	0.00
15.65	2.00	0.74	5.00	0.00	0.00	0.00
15.70 15.75	2.00	0.75 0.75	5.00 5.00	0.00 0.00	0.00 0.00	0.00
15.80	2.00	0.75	5.00	0.00	0.00	0.00
15.85	2.00	0.75	5.00	0.00	0.00	0.00
15.90	2.00	0.75	5.00	0.00	0.00	0.00
$\begin{array}{c} 15.95 \\ 16.00 \end{array}$	2.00	0.75 0.75	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
16.05	2.00	0.75	5.00	0.00	0.00	0.00
16.10	2.00	0.75	5.00	0.00	0.00	0.00
16.15 16.20	2.00	0.75 0.75	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
16.25	2.00	0.76	5.00	0.00	0.00	0.00
16.30	2.00	0.76	5.00	0.00	0.00	0.00
16.35 16.40	2.00	0.76 0.76	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
16.45	2.00	0.76	5.00	0.00	0.00	0.00
16.50	2.00	0.76	5.00	0.00	0.00	0.00
16.55 16.60	2.00	0.76 0.76	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
16.65	2.00	0.76	5.00	0.00	0.00	0.00
16.70	2.00	0.76	5.00	0.00	0.00	0.00
16.75 16.80	2.00	0.76 0.77	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00
16.85	2.00	0.77	5.00	0.00	0.00	0.00
16.90	2.00	0.77	5.00	0.00	0.00	0.00
16.95 17.00	2.00	0.77 0.77	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
17.05	2.00	0.77	5.00	0.00	0.00	0.00
17.10	2.00	0.77	5.00	0.00	0.00	0.00
17.15 17.20	2.00	0.77	5.00 5.00	$0.00 \\ 0.00$	0.00	0.00 0.00
17.25	2.00	0.77	5.00	0.00	0.00	0.00
17.30 17.35	2.00	0.77 0.78	5.00	0.00	0.00	$0.00 \\ 0.00$
17.33	2.00 2.00	0.78	5.00 5.00	0.00 0.00	0.00 0.00	0.00
17.45	2.00	0.78	5.00	0.00	0.00	0.00
17.50	2.00	0.78 0.78	5.00 5.00	$0.00 \\ 0.00$	0.00	0.00
17.55 17.60	2.00	0.78	5.00	0.00	0.00 0.00	0.00 0.00
17.65	2.00	0.78	5.00	0.00	0.00	0.00
$\frac{17.70}{17.75}$	2.00	0.78 0.78	5.00 5.00	0.00	0.00 0.00	$0.00 \\ 0.00$
17.75 17.80	2.00	0.78	5.00	$0.00 \\ 0.00$	0.00	0.00
17.85	2.00	0.78	5.00	0.00	0.00	0.00
17.90 17.95	2.00	0.78 0.78	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
18.00	2.00	0.79	5.00	0.00	0.00	0.00
18.05	2.00	0.79	5.00	0.00	0.00	0.00
18.10	2.00	0.79	5.00	0.00 Page 7	0.00	0.00

		C T-	116	67 6		
10 15	2 00			Class Coi		
18.15	2.00	0.79	5.00	0.00	0.00	0.00
18.20	2.00	0.79	5.00	0.00	0.00	0.00
18.25	2.00	0.79	5.00	0.00	0.00	0.00
18.30	2.00	0.79	5.00	0.00	0.00	0.00
18.35	2.00	0.79	5.00	0.00	0.00	0.00
18.40	2.00	0.79	5.00	0.00	0.00	0.00
18.45	2.00	0.79	5.00	0.00	0.00	0.00
18.50	2.00	0.79	5.00	0.00	0.00	0.00
18.55	2.00	0.79	5.00	0.00	0.00	0.00
18.60	2.00	0.79	5.00	0.00	0.00	0.00
18.65	2.00	0.80	5.00	0.00	0.00	0.00
18.70	2.00	0.80	5.00	0.00	0.00	0.00
18.75	2.00	0.80	5.00	0.00	0.00	0.00
18.80	2.00	0.80	5.00	0.00	0.00	0.00
18.85	2.00 2.00	0.80 0.80	5.00 5.00	0.00	0.00	0.00
18.90			5.00	0.00	0.00	0.00
18.95 19.00	2.00 2.00	$0.80 \\ 0.80$	5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 a a
19.05	2.00	0.80	5.00	0.00	0.00	0.00
19.10	2.00	0.80	5.00	0.00	0.00	0.00
19.15	2.00	0.80	5.00	0.00	0.00	0.00
19.20	2.00	0.80	5.00	0.00	0.00	0.00
19.25	2.00	0.80	5.00	0.00	0.00	0.00
19.30	2.00	0.81	5.00	0.00	0.00	0.00
19.35	2.00	0.81	5.00	0.00	0.00	0.00
19.40	2.00	0.81	5.00	0.00	0.00	0.00
19.45	2.00	0.81	5.00	0.00	0.00	0.00
19.50	2.00	0.81	5.00	0.00	0.00	0.00
19.55	2.00	0.81	5.00	0.00	0.00	0.00
19.60	2.00	0.81	5.00	0.00	0.00	0.00
19.65	2.00	0.81	5.00	0.00	0.00	0.00
19.70	2.00	0.81	5.00	0.00	0.00	0.00
19.75	2.00	0.81	5.00	0.00	0.00	0.00
19.80	2.00	0.81	5.00	0.00	0.00	0.00
19.85	2.00	0.81	5.00	0.00	0.00	0.00
19.90	2.00	0.81	5.00	0.00	0.00	0.00
19.95	2.00	0.81	5.00	0.00	0.00	0.00
20.00	2.00	0.81	5.00	0.00	0.00	0.00
20.05	2.00	0.82	5.00	0.00	0.00	0.00
20.10	2.00	0.82	5.00	0.00	0.00	0.00
20.15	2.00	0.82	5.00	0.00	0.00	0.00
20.20	2.00	0.82	5.00	0.00	0.00	0.00
20.25	2.00	0.82	5.00	0.00	0.00	0.00
20.30	2.00	0.82	5.00	0.00	0.00	0.00
20.35	2.00	0.82	5.00	0.00	0.00	0.00
20.40	2.00	0.82	5.00	0.00	0.00	0.00
20.45	2.00	0.82	5.00	0.00	0.00	0.00
20.50 20.55	2.00	0.82	5.00	0.00	0.00	0.00
20.55	2.00	0.82	5.00 5.00	0.00	0.00	0.00
20.60	2.00	0.82 0.82	5.00	0.00 0.00	0.00 0.00	0.00 0.00
20.65 20.70	2.00 2.00	0.82	5.00	0.00	0.00	0.00
20.75	2.00	0.82	5.00	0.00	0.00	0.00
20.80	2.00	0.82	5.00	0.00	0.00	0.00
20.85	2.00	0.83	5.00	0.00	0.00	0.00
20.90	2.00	0.83	5.00	0.00	0.00	0.00
20.95	2.00	0.83	5.00	0.00	0.00	0.00
21.00	2.00	0.83	5.00	0.00	0.00	0.00
21.05	2.00	0.83	5.00	0.00	0.00	0.00
21.10	2.00	0.83	5.00	0.00	0.00	0.00
21.15	2.00	0.83	5.00	0.00	0.00	0.00
21.20	2.00	0.83	5.00	0.00	0.00	0.00
21.25	2.00	0.83	5.00	0.00	0.00	0.00
				Dage 9		

		Santa To	roca UC	Class Co	ncassion	ER1 cum
21.30	2.00	0.83	5.00	0.00	0.00	0.00
21.35	2.00	0.83	5.00	0.00	0.00	0.00
21.40	2.00	0.83	5.00	0.00	0.00	0.00
21.45 21.50	2.00	0.83 0.83	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
21.55	2.00	0.83	5.00	0.00	0.00	0.00
21.60	2.00	0.83	5.00	0.00	0.00	0.00
21.65	2.00	0.84	5.00	0.00	0.00	0.00
21.70 21.75	2.00	0.84 0.84	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00
21.73	2.00	0.84	5.00	0.00	0.00	0.00
21.85	2.00	0.84	5.00	0.00	0.00	0.00
21.90	2.00	0.84	5.00	0.00	0.00	0.00
21.95 22.00	2.00 2.00	0.84 0.84	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00
22.05	2.00	0.84	5.00	0.00	0.00	0.00
22.10	2.00	0.84	5.00	0.00	0.00	0.00
22.15	2.00	0.84	5.00	0.00	0.00	0.00
22.20 22.25	2.00	0.84 0.84	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00
22.30	2.00	0.84	5.00	0.00	0.00	0.00
22.35	2.00	0.84	5.00	0.00	0.00	0.00
22.40 22.45	2.00	0.84 0.84	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00
22.50	2.00	0.85	5.00	0.00	0.00	0.00
22.55	2.00	0.85	5.00	0.00	0.00	0.00
22.60	2.00	0.85	5.00	0.00	0.00	0.00
22.65 22.70	2.00	0.85 0.85	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$
22.75	2.00	0.85	5.00	0.00	0.00	0.00
22.80	2.00	0.85	5.00	0.00	0.00	0.00
22.85 22.90	2.00 2.00	0.85 0.85	5.00 5.00	$0.00 \\ 0.00$	0.00	0.00
22.95	2.00	0.85	5.00	0.00	0.00	0.00
23.00	2.00	0.85	5.00	0.00	0.00	0.00
23.05	2.00	0.85	5.00	0.00	0.00	0.00
23.10 23.15	2.00	0.85 0.85	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00
23.20	2.00	0.85	5.00	0.00	0.00	0.00
23.25	2.00	0.85	5.00	0.00	0.00	0.00
23.30 23.35	2.00	0.85 0.85	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
23.40	2.00	0.86	5.00	0.00	0.00	0.00
23.45	2.00	0.86	5.00	0.00	0.00	0.00
23.50	2.00	0.86	5.00	0.00	0.00	0.00
23.55 23.60	2.00	0.86 0.86	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
23.65	2.00	0.86	5.00	0.00	0.00	0.00
23.70	2.00	0.86	5.00	0.00	0.00	0.00
23.75 23.80	2.00	0.86 0.86	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
23.85	2.00	0.86	5.00	0.00	0.00	0.00
23.90	2.00	0.86	5.00	0.00	0.00	0.00
23.95	2.00	0.86	5.00	0.00	0.00	0.00
24.00 24.05	2.00 2.00	0.86 0.86	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
24.10	2.00	0.86	5.00	0.00	0.00	0.00
24.15	2.00	0.86	5.00	0.00	0.00	0.00
24.20 24.25	2.00	0.86 0.86	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$
24.23	2.00	0.86	5.00	0.00	0.00	0.00
24.35	2.00	0.87	5.00	0.00	0.00	0.00
24.40	2.00	0.87	5.00	0.00	0.00	0.00

		Santa Te	resa HS	Class Cor	ncession	EB1.sum
24.45	2.00	0.87	5.00	0.00	0.00	0.00
24.50	2.00	0.87	5.00	0.00	0.00	0.00
24.55	2.00	0.87	5.00	0.00	0.00	0.00
24.60	2.00	0.87	5.00	0.00	0.00	0.00
24.65	2.00	0.87	5.00	0.00	0.00	0.00
24.70	2.00	0.87	5.00	0.00	0.00	0.00
24.75 24.80	2.00 2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00
24.85	2.00	0.87	5.00	0.00	0.00	0.00
24.90	2.00	0.87	5.00	0.00	0.00	0.00
24.95	2.00	0.87	5.00	0.00	0.00	0.00
25.00	2.00	0.87	5.00	0.00	0.00	0.00
25.05	2.00	0.87	5.00	0.00	0.00	0.00
25.10	2.00	0.87	5.00	0.00	0.00	0.00
25.15	2.00	0.87	5.00	0.00	0.00	0.00
25.20	2.00	0.87	5.00	0.00	0.00	0.00
25.25 25.30	2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
25.35	2.00	0.87	5.00	0.00	0.00	0.00
25.40	2.00	0.88	5.00	0.00	0.00	0.00
25.45	2.00	0.88	5.00	0.00	0.00	0.00
25.50	2.00	0.88	5.00	0.00	0.00	0.00
25.55	2.00	0.88	5.00	0.00	0.00	0.00
25.60	2.00	0.88	5.00	0.00	0.00	0.00
25.65	2.00	0.88	5.00	0.00	0.00	0.00
25.70	2.00	0.88	5.00	0.00	0.00 0.00	0.00
25.75 25.80	2.00	$0.88 \\ 0.88$	5.00 5.00	0.00	0.00	0.00 0.00
25.85	2.00	0.88	5.00	0.00	0.00	0.00
25.90	2.00	0.88	5.00	0.00	0.00	0.00
25.95	2.00	0.88	5.00	0.00	0.00	0.00
26.00	2.00	0.88	5.00	0.00	0.00	0.00
26.05	2.00	0.88	5.00	0.00	0.00	0.00
26.10	2.00	0.88	5.00	0.00	0.00	0.00
26.15	2.00	0.88	5.00	0.00	0.00	0.00
26.20 26.25	2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
26.30	2.00 2.00	0.88	5.00	0.00	0.00	0.00
26.35	2.00	0.88	5.00	0.00	0.00	0.00
26.40	2.00	0.88	5.00	0.00	0.00	0.00
26.45	2.00	0.88	5.00	0.00	0.00	0.00
26.50	2.00	0.88	5.00	0.00	0.00	0.00
26.55	2.00	0.88	5.00	0.00	0.00	0.00
26.60	2.00	0.89	5.00	0.00	0.00	0.00
26.65 26.70	2.00 2.00	0.89 0.89	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
26.75	2.00	0.89	5.00	0.00	0.00	0.00
26.80	2.00	0.89	5.00	0.00	0.00	0.00
26.85	2.00	0.89	5.00	0.00	0.00	0.00
26.90	2.00	0.89	5.00	0.00	0.00	0.00
26.95	2.00	0.89	5.00	0.00	0.00	0.00
27.00	2.00	0.89	5.00	0.00	0.00	0.00
27.05	2.00	0.89	5.00	0.00	0.00	0.00
27.10 27.15	2.00	0.89 0.89	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
27.20	2.00	0.89	5.00	0.00	0.00	0.00
27.25	2.00	0.89	5.00	0.00	0.00	0.00
27.30	2.00	0.89	5.00	0.00	0.00	0.00
27.35	2.00	0.89	5.00	0.00	0.00	0.00
27.40	2.00	0.89	5.00	0.00	0.00	0.00
27.45	2.00	0.89	5.00	0.00	0.00	0.00
27.50 27.55	2.00	0.89	5.00	0.00	0.00	$0.00 \\ 0.00$
27.33	2.00	0.89	5.00	0.00 Page 10	0.00	0.00

		Santa Te	resa HS	class Cor	ncession	EB1.sum
27.60	2.00	0.89	5.00	0.00	0.00	0.00
27.65 27.70	2.00	$0.89 \\ 0.89$	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
27.75	2.00	0.89	5.00	0.00	0.00	0.00
27.80	2.00	0.89	5.00	0.00	0.00	0.00
27.85	2.00	0.89	5.00	0.00	0.00	0.00
27.90	2.00	0.89 0.89	5.00 5.00	0.00	$0.00 \\ 0.00$	0.00 0.00
27.95 28.00	2.00 2.00	0.89	5.00	0.00 0.00	0.00	0.00
28.05	2.00	0.89	5.00	0.00	0.00	0.00
28.10	2.00	0.90	5.00	0.00	0.00	0.00
28.15 28.20	2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
28.25	2.00	0.90	5.00	0.00	0.00	0.00
28.30	2.00	0.90	5.00	0.00	0.00	0.00
28.35	2.00	0.90	5.00	0.00	0.00	0.00
28.40 28.45	2.00 2.00	0.90 0.90	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
28.50	2.00	0.90	5.00	0.00	0.00	0.00
28.55	2.00	0.90	5.00	0.00	0.00	0.00
28.60 28.65	2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00
28.70	2.00	0.90	5.00	0.00	0.00	0.00
28.75	2.00	0.90	5.00	0.00	0.00	0.00
28.80	2.00	0.90	5.00	0.00	0.00	0.00
28.85 28.90	2.00	0.90 0.90	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
28.95	2.00	0.90	5.00	0.00	0.00	0.00
29.00	2.00	0.90	5.00	0.00	0.00	0.00
29.05 29.10	2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
29.15	2.00	0.90	5.00	0.00	0.00	0.00
29.15 29.20	2.00	0.90	5.00	0.00	0.00	0.00
29.25	2.00	0.90	5.00	0.00	0.00	0.00
29.30 29.35	2.00 2.00	$0.90 \\ 0.90$	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
29.40	2.00	0.90	5.00	0.00	0.00	0.00
29.45	2.00	0.90	5.00	0.00	0.00	0.00
29.50 29.55	2.00	$0.90 \\ 0.90$	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
29.60	2.00	0.90	5.00	0.00	0.00	0.00
29.65	2.00	0.90	5.00	0.00	0.00	0.00
29.70 29.75	2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
29.80	2.00	0.90	5.00	0.00	0.00	0.00
29.85	2.00	0.90	5.00	0.00	0.00	0.00
29.90 29.95	2.00	0.90	5.00 5.00	$0.00 \\ 0.00$	0.00	$0.00 \\ 0.00$
30.00	2.00	0.90 0.90	5.00	0.00	0.00 0.00	0.00
30.05	2.00	0.90	5.00	0.00	0.00	0.00
30.10	2.00	0.90	5.00	0.00	0.00	0.00
30.15 30.20	2.00	$0.90 \\ 0.90$	5.00 5.00	0.00 0.00	0.00	$0.00 \\ 0.00$
30.25	2.00	0.90	5.00	0.00	0.00	0.00
30.30	2.00	0.90	5.00	0.00	0.00	0.00
30.35 30.40	2.00	$0.90 \\ 0.90$	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
30.45	2.00	0.90	5.00	0.00	0.00	0.00
30.50	2.00	0.90	5.00	0.00	0.00	0.00
30.55	2.00	0.90	5.00	0.00	0.00	0.00 0.00
30.60 30.65	2.00	$0.90 \\ 0.90$	5.00 5.00	0.00 0.00	0.00 0.00	0.00
30.70	2.00	0.90	5.00	0.00	0.00	0.00
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		Santa Te	resa HS	class Con	cession	EB1.sum
30.75	2.00	0.90	5.00	0.00	0.00	0.00
30.80	2.00	0.90	5.00	0.00	0.00	0.00
30.85 30.90	2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
30.95	2.00	0.90	5.00	0.00	0.00	0.00
31.00	2.00	0.90	5.00	0.00	0.00	0.00
31.05	2.00	0.90	5.00	0.00	0.00	0.00
31.10	2.00	0.90	5.00	0.00	0.00	0.00
31.15 31.20	2.00 2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
31.25	2.00	0.90	5.00	0.00	0.00	0.00
31.30	2.00	0.90	5.00	0.00	0.00	0.00
31.35	2.00	0.90	5.00	0.00	0.00	0.00
31.40 31.45	2.00	0.90 0.90	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
31.50	2.00	0.90	5.00	0.00	0.00	0.00
31.55	2.00	0.90	5.00	0.00	0.00	0.00
31.60	2.00	0.90	5.00	0.00	0.00	0.00
31.65	2.00	0.90	5.00 5.00	0.00	0.00	0.00
31.70 31.75	2.00 2.00	$0.90 \\ 0.90$	5.00	0.00 0.00	0.00 0.00	0.00 0.00
31.80	2.00	0.90	5.00	0.00	0.00	0.00
31.85	2.00	0.90	5.00	0.00	0.00	0.00
31.90	2.00	0.90	5.00	0.00	0.00	0.00
31.95 32.00	2.00	$0.90 \\ 0.90$	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
32.05	2.00	0.90	5.00	0.00	0.00	0.00
32.10	2.00	0.90	5.00	0.00	0.00	0.00
32.15	2.00	0.90	5.00	0.00	0.00	0.00
32.20 32.25	2.00	0.90 0.90	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
32.30	2.00	0.90	5.00	0.00	0.00	0.00
32.35	2.00	0.90	5.00	0.00	0.00	0.00
32.40	2.00	0.90	5.00	0.00	0.00	0.00
32.45 32.50	2.00 2.00	$0.90 \\ 0.90$	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
32.55	2.00	0.90	5.00	0.00	0.00	0.00
32.60	2.00	0.90	5.00	0.00	0.00	0.00
32.65	2.00	0.90	5.00	0.00	0.00	0.00
32.70 32.75	2.00	0.90 0.90	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
32.80	2.00	0.90	5.00	0.00	0.00	0.00
32.85	2.00	0.90	5.00	0.00	0.00	0.00
32.90	2.00	0.90	5.00 5.00	0.00 0.00	0.00	0.00 0.00
32.95 33.00	2.00	$0.90 \\ 0.90$	5.00	0.00	0.00	0.00
33.05	2.00	0.90	5.00	0.00	0.00	0.00
33.10	2.00	0.90	5.00	0.00	0.00	0.00
33.15	2.00	0.90	5.00	0.00	0.00	0.00 0.00
33.20 33.25	2.00	0.90 0.90	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00
33.30	2.00	0.90	5.00	0.00	0.00	0.00
33.35	2.00	0.90	5.00	0.00	0.00	0.00
33.40	2.00	0.90	5.00	0.00	0.00	0.00
33.45 33.50	2.00	0.90 0.90	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
33.55	2.00	0.90	5.00	0.00	0.00	0.00
33.60	2.00	0.90	5.00	0.00	0.00	0.00
33.65	2.00	0.90 0.90	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
33.70 33.75	2.00	0.90	5.00	0.00	0.00	0.00
33.80	2.00	0.90	5.00	0.00	0.00	0.00
33.85	2.00	0.90	5.00	0.00	0.00	0.00
				Page 12		

		Santa Te	resa HS	Class Co	ncession	EB1.sum
33.90	2.00	0.90	5.00	0.00	0.00	0.00
33.95	2.00	0.90	5.00	0.00	0.00	0.00
34.00	2.00	0.90	5.00	0.00	0.00	0.00
34.05	2.00	0.90	5.00	0.00	0.00	0.00
34.10	2.00	0.90	5.00 5.00	0.00	0.00	0.00
34.15 34.20	2.00	0.90 0.90	5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00
34.25	2.00	0.90	5.00	0.00	0.00	0.00
34.30	2.00	0.90	5.00	0.00	0.00	0.00
34.35	2.00	0.90	5.00	0.00	0.00	0.00
34.40	2.00	0.90	5.00	0.00	0.00	0.00
34.45	2.00	0.90	5.00	0.00	0.00	0.00
34.50	2.00	0.90	5.00	0.00	0.00	0.00
34.55 34.60	2.00 2.00	0.90 0.90	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
34.65	2.00	0.90	5.00	0.00	0.00	0.00
34.70	2.00	0.90	5.00	0.00	0.00	0.00
34.75	2.00	0.89	5.00	0.00	0.00	0.00
34.80	2.00	0.89	5.00	0.00	0.00	0.00
34.85	2.00	0.89	5.00	0.00	0.00	0.00
34.90	2.00	0.89	5.00	0.00	0.00	0.00
34.95	2.00 2.00	0.89	5.00 5.00	0.00	0.00	0.00
35.00 35.05	2.00	$0.89 \\ 0.89$	5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	0.00 0.00
35.10	2.00	0.89	5.00	0.00	0.00	0.00
35.15	2.00	0.89	5.00	0.00	0.00	0.00
35.20	2.00	0.89	5.00	0.00	0.00	0.00
35.25	2.00	0.89	5.00	0.00	0.00	0.00
35.30	2.00	0.89	5.00	0.00	0.00	0.00
35.35	2.00 2.00	$0.89 \\ 0.89$	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
35.40 35.45	2.00	0.89	5.00	0.00	0.00	0.00
35.50	2.00	0.00	5.00	0.00	0.00	0.00
35.55	2.00	0.89	5.00	0.00	0.00	0.00
35.60	2.00	0.89	5.00	0.00	0.00	0.00
35.65	2.00	0.89	5.00	0.00	0.00	0.00
35.70	2.00	0.89	5.00	0.00	0.00	0.00
35.75 35.80	2.00	$0.89 \\ 0.89$	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
35.85	2.00	0.89	5.00	0.00	0.00	0.00
35.90	2.00	0.89	5.00	0.00	0.00	0.00
35.95	2.00	0.89	5.00	0.00	0.00	0.00
36.00	2.00	0.89	5.00	0.00	0.00	0.00
36.05	2.00	0.89	5.00	0.00	0.00	0.00
36.10	2.00	0.89	5.00	0.00	0.00	0.00
36.15 36.20	2.00	0.89 0.89	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00
36.25	2.00	0.89	5.00	0.00	0.00	0.00
36.30	2.00	0.89	5.00	0.00	0.00	0.00
36.35	2.00	0.89	5.00	0.00	0.00	0.00
36.40	2.00	0.89	5.00	0.00	0.00	0.00
36.45	2.00	0.89	5.00	0.00	0.00	0.00
36.50	2.00	0.89	5.00	0.00	0.00	0.00
36.55	2.00	0.89	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
36.60 36.65	2.00	0.89 0.89	5.00	0.00	0.00	0.00
36.70	2.00	0.89	5.00	0.00	0.00	0.00
36.75	2.00	0.89	5.00	0.00	0.00	0.00
36.80	2.00	0.89	5.00	0.00	0.00	0.00
36.85	2.00	0.89	5.00	0.00	0.00	0.00
36.90	2.00	0.89	5.00	0.00	0.00	0.00
36.95 37.00	2.00	0.89 0.89	5.00 5.00	0.00	0.00 0.00	$0.00 \\ 0.00$
37.00	2.00	0.09	3.00	0.00	0.00	0.00

		Santa Te	resa HS	Class Cor	ncession	EB1.sum
37.05	2.00	0.89	5.00	0.00	0.00	0.00
37.10	2.00	0.89	5.00	0.00	0.00	0.00
37.15	2.00	0.89	5.00	0.00	0.00	0.00
37.20 37.25	2.00 2.00	$0.89 \\ 0.89$	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$
37.30	2.00	0.89	5.00	0.00	0.00	0.00
37.35	2.00	0.89	5.00	0.00	0.00	0.00
37.40	2.00	0.89	5.00	0.00	0.00	0.00
37.45	2.00	0.89	5.00	0.00	0.00	0.00
37.50 37.55	2.00	$0.89 \\ 0.89$	5.00 5.00	$0.00 \\ 0.00$	0.00	0.00
37.60	2.00	0.89	5.00	0.00	$0.00 \\ 0.00$	0.00
37.65	2.00	0.89	5.00	0.00	0.00	0.00
37.70	2.00	0.89	5.00	0.00	0.00	0.00
37.75	2.00	0.89	5.00	0.00	0.00	0.00
37.80 37.85	2.00 2.00	$0.89 \\ 0.89$	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00 0.00
37.90	2.00	0.89	5.00	0.00	0.00	0.00
37.95	2.00	0.89	5.00	0.00	0.00	0.00
38.00	2.00	0.89	5.00	0.00	0.00	0.00
38.05 38.10	2.00	0.89 0.88	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00
38.15	2.00	0.88	5.00	0.00	0.00	0.00
38.20	2.00	0.88	5.00	0.00	0.00	0.00
38.25	2.00	0.88	5.00	0.00	0.00	0.00
38.30	2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00	0.00
38.35 38.40	2.00	0.88	5.00	0.00	$0.00 \\ 0.00$	0.00 0.00
38.45	2.00	0.88	5.00	0.00	0.00	0.00
38.50	2.00	0.88	5.00	0.00	0.00	0.00
38.55	2.00	0.88	5.00	0.00	0.00	0.00
38.60 38.65	2.00	0.88 0.88	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
38.70	2.00	0.88	5.00	0.00	0.00	0.00
38.75	2.00	0.88	5.00	0.00	0.00	0.00
38.80	2.00	0.88	5.00	0.00	0.00	0.00
38.85 38.90	2.00	$0.88 \\ 0.88$	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
38.95	2.00	0.88	5.00	0.00	0.00	0.00
39.00	2.00	0.88	5.00	0.00	0.00	0.00
39.05	2.00	0.88	5.00	0.00	0.00	0.00
39.10 39.15	2.00	0.88 0.88	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
39.20	2.00	0.88	5.00	0.00	0.00	0.00
39.25	2.00	0.88	5.00	0.00	0.00	0.00
39.30	2.00	0.88	5.00	0.00	0.00	0.00
39.35 39.40	2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
39.45	2.00	0.88	5.00	0.00	0.00	0.00
39.50	2.00	0.88	5.00	0.00	0.00	0.00
39.55	2.00	0.88	5.00	0.00	0.00	0.00
39.60 39.65	2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
39.70	2.00	0.88	5.00	0.00	0.00	0.00
39.75	2.00	0.88	5.00	0.00	0.00	0.00
39.80	2.00	0.88	5.00	0.00	0.00	0.00
39.85 39.90	2.00	$0.88 \\ 0.88$	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
39.95	2.00	0.88	5.00	0.00	0.00	0.00
40.00	2.00	0.88	5.00	0.00	0.00	0.00
40.05	2.00	0.88	5.00	0.00	0.00	0.00
40.10	2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
40.15	2.00	0.00	5.00	Dags 14	0.00	0.00

		Santa Te	resa HS	class Cor	cession	EB1.sum
40.20	2.00	0.88	5.00	0.00	0.00	0.00
40.25	2.00	0.88	5.00	0.00	0.00	0.00
40.30 40.35	2.00	0.88 0.88	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
40.40	2.00	0.88	5.00	0.00	0.00	0.00
40.45	2.00	0.88	5.00	0.00	0.00	0.00
40.50	2.00	0.88	5.00	0.00	0.00	0.00
40.55 40.60	2.00	0.88 0.88	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
40.65	2.00	0.88	5.00	0.00	0.00	0.00
40.70	2.00	0.88	5.00	0.00	0.00	0.00
40.75	2.00	0.87	5.00	0.00	0.00	0.00
40.80 40.85	2.00 2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
40.90	2.00	0.87	5.00	0.00	0.00	0.00
40.95	2.00	0.87	5.00	0.00	0.00	0.00
41.00 41.05	2.00 2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
41.10	2.00	0.87	5.00	0.00	0.00	0.00
41.15	2.00	0.87	5.00	0.00	0.00	0.00
41.20	2.00	0.87	5.00	0.00	0.00	0.00
41.25 41.30	2.00 2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
41.35	2.00	0.87	5.00	0.00	0.00	0.00
41.40	2.00	0.87	5.00	0.00	0.00	0.00
41.45 41.50	2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	$0.00 \\ 0.00$	$0.00 \\ 0.00$
41.55	2.00	0.87	5.00	0.00	0.00	0.00
41.60	2.00	0.87	5.00	0.00	0.00	0.00
41.65	2.00	0.87	5.00	0.00	0.00	0.00
41.70 41.75	2.00 2.00	0.87 0.87	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
41.80	2.00	0.87	5.00	0.00	0.00	0.00
41.85	2.00	0.87	5.00	0.00	0.00	0.00
41.90	2.00	0.87	5.00	0.00	0.00	0.00
41.95 42.00	2.00	0.87 0.87	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
42.05	2.00	0.87	5.00	0.00	0.00	0.00
42.10	2.00	0.87	5.00	0.00	0.00	0.00
42.15 42.20	2.00	0.87 0.87	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
42.25	2.00	0.87	5.00	0.00	0.00	0.00
42.30	2.00	0.87	5.00	0.00	0.00	0.00
42.35	2.00	0.87	5.00 5.00	0.00	0.00	0.00
42.40 42.45	2.00	0.87 0.87	5.00	0.00	0.00 0.00	$0.00 \\ 0.00$
42.50	2.00	0.87	5.00	0.00	0.00	0.00
42.55	2.00	0.87	5.00	0.00	0.00	0.00
42.60 42.65	2.00 2.00	0.87 0.87	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	0.00
42.70	2.00	0.87	5.00	0.00	0.00	0.00
42.75	2.00	0.87	5.00	0.00	0.00	0.00
42.80 42.85	2.00	0.87	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
42.83	2.00	0.87 0.87	5.00	0.00	0.00	0.00
42.95	2.00	0.86	5.00	0.00	0.00	0.00
43.00	2.00	0.86	5.00	0.00	0.00	0.00
43.05 43.10	2.00	0.86 0.86	5.00 5.00	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
43.15	2.00	0.86	5.00	0.00	0.00	0.00
43.20	2.00	0.86	5.00	0.00	0.00	0.00
43.25 43.30	2.00	0.86 0.86	5.00 5.00	$0.00 \\ 0.00$	0.00 0.00	$0.00 \\ 0.00$
73.30	2.00	0.00	3.00	Page 15	0.00	0.00

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Santa Teresa HS Class Concession EB1.sum
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43.35
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* F.S.<1, Liquefaction Potential Zone
(F.S. is limited to 5, CRR is limited to 2, CSR is limited to 2)

Units: Unit: qc, fs, Stress or Pressure = atm (1.0581tsf); Unit Weight = pcf; Depth = ft; Settlement = in.

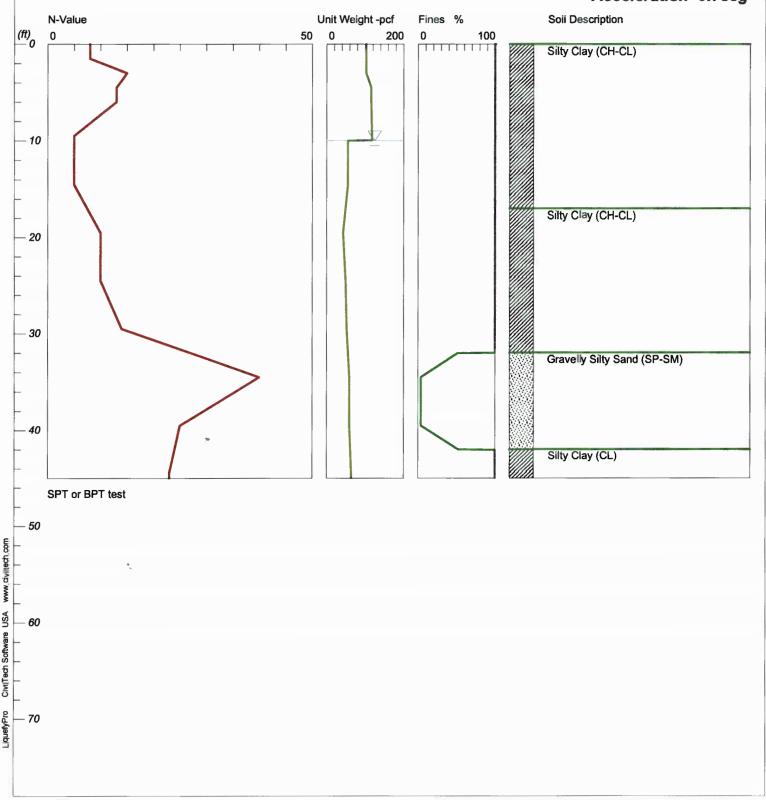
```
1 atm (atmosphere) = 1 tsf (ton/ft2)
CRRM Cyclic resistance ratio from soils
CSRsf Cyclic stress ratio induced by a given earthquake (with user request factor of safety)
F.S. Factor of Safety against liquefaction, F.S.=CRRm/CSRsf
S_sat Settlement from saturated sands
S_dry Settlement from Unsaturated Sands
S_all Total Settlement from Saturated and Unsaturated Sands
NO-Liquefy Soils
```

LIQUEFACTION ANALYSIS

Santa Teresa HS Classroom and Concession Buildings

Hole No.=EB-4 Water Depth=10 ft

Magnitude=8.5 Acceleration=0.735g

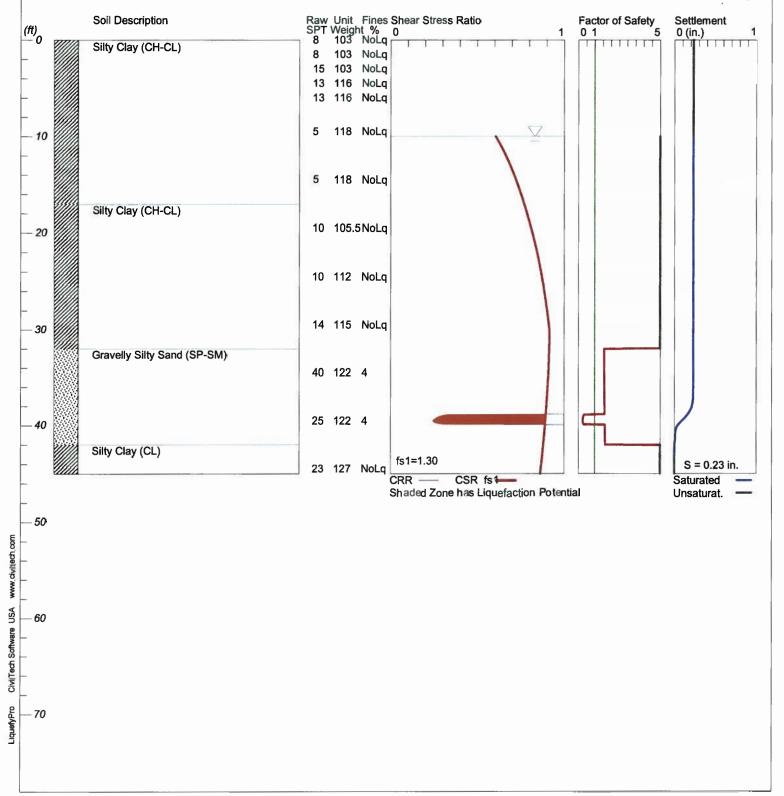


LIQUEFACTION ANALYSIS

Santa Teresa HS Classroom and Concession Buildings

Hole No.=EB-4 Water Depth=10 ft

Magnitude=8.5 Acceleration=0.735g



Santa Teresa HS Class Concession EB4.sum

************************** ****** LIQUEFACTION ANALYSIS SUMMARY Copyright by CivilTech Software www.civiltechsoftware.com ***************** *********** Font: Courier New, Regular, Size 8 is recommended for this report. Licensed to , 12/4/2015 2:16:39 PM Licensed to . Input File Name: C:\Grant Roughs\Liquefy Pro Data Files\Santa Teresa HS Class Concession EB4.liq Title: Santa Teresa HS Classroom and Concession Buildings Subtitle: Surface Elev.= Hole No.=EB-4 Depth of Hole= 45.00 ft Water Table during Earthquake= 10.00 ft Water Table during In-Situ Testing= 10.00 ft Max. Acceleration= 0.74 g Earthquake Magnitude= 8.50 Input Data: Surface Elev.= Hole No.=EB-4 Depth of Hole=45.00 ft Water Table during Earthquake= 10.00 ft Water Table during In-Situ Testing= 10.00 ft Max. Acceleration=0.74 g Earthquake Magnitude=8.50 No-Liquefiable Soils: CL, OL are Non-Liq. Soil SPT or BPT Calculation. 2. Settlement Analysis Method: Tokimatsu, M-correction 3. Fines Correction for Liquefaction: Idriss/Seed
4. Fine Correction for Settlement: During Liquefaction*
5. Settlement Calculation in: All zones*
6. Hammer Energy Ratio, Ce = 1.257. Borehole Diameter, Cb=1Sampling Method, Cs=19. User request factor of safety (apply to CSR), User= 1.3 Plot one CSR curve (fs1=User) 10. Use Curve Smoothing: Yes* * Recommended Options

	Test Da SPT	ta: gamma pcf	Fines %
0.00	8.00	103.00	NoLiq
1.50	8.00	103.00	NoLiq
3.00	15.00	103.00	NoLiq
4.50	13.00	116.00	NoLiq
6.00	13.00	116.00	NoLiq
9.50	5.00	118.00	NoLiq
14.50	5.00	118.00	NoLiq
19.50	10.00	105.50	NoLiq
24.50	10.00	112.00	NoLiq

Santa Teresa HS Class Concession EB4.sum

29.50	14.00	115.00	NoLiq	
34.50	40.00	122.00	4.00	
39.50	25.00	122.00	4.00	
44.50	23.00	127.00	NoLiq	
			_	

Output Results:
Settlement of Saturated Sands=0.23 in.
Settlement of Unsaturated Sands=0.00 in.
Total Settlement of Saturated and Unsaturated Sands=0.23 in.
Differential Settlement=0.116 to 0.153 in.

Depth ft	CRRM	CSRfs	F.S.	S_sat. in.	S_dry in.	S_all in.
0.00 0.05 0.10 0.15 0.20 0.35 0.40 0.55 0.65 0.75 0.80 0.95 1.05 1.15 1.20 1.35 1.45 1.55 1.65 1.75 1.85 1.95 2.20 2.25 2.35	2.00 2.00 2.00 2.00 2.00 2.00 2.00 2.00	0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62	5.00 5.00	0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23	0.00 0.00	0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23

Santa Teresa HS Class Concession EB4.	SIIM
2.40 2.00 0.62 5.00 0.23 0.00 0.2	3
2.45 2.00 0.62 5.00 0.23 0.00 0.2	3
2.50 2.00 0.62 5.00 0.23 0.00 0.2	
2.55	ე ვ
2.65 2.00 0.62 5.00 0.23 0.00 0.2 2.65 2.00 0.62 5.00 0.23 0.00 0.2	3
2.70 2.00 0.62 5.00 0.23 0.00 0.2	3
2.75 2.00 0.62 5.00 0.23 0.00 0.2	3
2.80 2.00 0.62 5.00 0.23 0.00 0.2	3
2.85 2.00 0.62 5.00 0.23 0.00 0.2 2.90 2.00 0.62 5.00 0.23 0.00 0.2	ქ 2
2.95 2.00 0.62 5.00 0.23 0.00 0.2	ე ვ
3.00 2.00 0.62 5.00 0.23 0.00 0.2	<u>3</u>
3.05 2.00 0.62 5.00 0.23 0.00 0.2	3
3.10 2.00 0.62 5.00 0.23 0.00 0.2	3
3.15 2.00 0.62 5.00 0.23 0.00 0.2 3.20 2.00 0.62 5.00 0.23 0.00 0.2	3 3
3.25 2.00 0.62 5.00 0.23 0.00 0.2	3
3.30 2.00 0.62 5.00 0.23 0.00 0.2	3
3.35 2.00 0.62 5.00 0.23 0.00 0.2	3
3.40 2.00 0.62 5.00 0.23 0.00 0.2 3.45 2.00 0.62 5.00 0.23 0.00 0.2	3 3
3.50 2.00 0.62 5.00 0.23 0.00 0.2	3
3.55 2.00 0.62 5.00 0.23 0.00 0.2	3
3.60 2.00 0.62 5.00 0.23 0.00 0.2 3.65 2.00 0.62 5.00 0.23 0.00 0.2	3
3.65 2.00 0.62 5.00 0.23 0.00 0.2 3.70 2.00 0.62 5.00 0.23 0.00 0.2	3 3
3.75 2.00 0.62 5.00 0.23 0.00 0.2	3
3.80 2.00 0.62 5.00 0.23 0.00 0.2	3
3.85 2.00 0.62 5.00 0.23 0.00 0.2	3
3.90 2.00 0.62 5.00 0.23 0.00 0.2 3.95 2.00 0.62 5.00 0.23 0.00 0.2	პ ვ
4.00 2.00 0.62 5.00 0.23 0.00 0.2	3
4.05 2.00 0.62 5.00 0.23 0.00 0.2	3
4.10 2.00 0.62 5.00 0.23 0.00 0.2	3
4.15 2.00 0.62 5.00 0.23 0.00 0.2 4.20 2.00 0.61 5.00 0.23 0.00 0.2	<u>ქ</u>
4.25 2.00 0.61 5.00 0.23 0.00 0.2	3
4.30 2.00 0.61 5.00 0.23 0.00 0.2	3
4.35 2.00 0.61 5.00 0.23 0.00 0.2 4.40 2.00 0.61 5.00 0.23 0.00 0.2	3
4.40 2.00 0.61 5.00 0.23 0.00 0.2 4.45 2.00 0.61 5.00 0.23 0.00 0.2	3 2
4.50 2.00 0.61 5.00 0.23 0.00 0.2	3
4.55 2.00 0.61 5.00 0.23 0.00 0.2	3
4.60 2.00 0.61 5.00 0.23 0.00 0.2	3
4.65 2.00 0.61 5.00 0.23 0.00 0.2 4.70 2.00 0.61 5.00 0.23 0.00 0.2	3 3
4.75 2.00 0.61 5.00 0.23 0.00 0.2	3
4.80 2.00 0.61 5.00 0.23 0.00 0.2	3
4.85 2.00 0.61 5.00 0.23 0.00 0.2	3
4.90 2.00 0.61 5.00 0.23 0.00 0.2 4.95 2.00 0.61 5.00 0.23 0.00 0.2	3
4.95 2.00 0.61 5.00 0.23 0.00 0.2 5.00 2.00 0.61 5.00 0.23 0.00 0.2	3
5.05 2.00 0.61 5.00 0.23 0.00 0.2	3
5.10 2.00 0.61 5.00 0.23 0.00 0.2 5.15 2.00 0.61 5.00 0.23 0.00 0.2	3
5.15 2.00 0.61 5.00 0.23 0.00 0.2 5.20 2.00 0.61 5.00 0.23 0.00 0.2	3
5.25 2.00 0.61 5.00 0.23 0.00 0.2	3
5.30 2.00 0.61 5.00 0.23 0.00 0.2	3
5.35 2.00 0.61 5.00 0.23 0.00 0.2	5
5.40 2.00 0.61 5.00 0.23 0.00 0.2 5.45 2.00 0.61 5.00 0.23 0.00 0.2	3
5.50 2.00 0.61 5.00 0.23 0.00 0.2	

		Santa To	raca US	class Cor	cession	FR4 CUM
5.55	2.00	0.61	5.00	0.23	0.00	0.23
5.60	2.00	0.61	5.00	0.23	0.00	0.23
5.65	2.00	0.61	5.00	0.23	0.00	0.23
5.70	2.00	0.61	5.00	0.23	0.00	0.23
5.75	2.00	0.61	5.00	0.23	0.00	0.23
5.80	2.00	0.61	5.00	0.23	0.00	0.23
5.85	2.00	0.61	5.00	0.23	0.00	0.23
5.90	2.00	0.61	5.00	0.23	0.00	0.23
5.95	2.00	0.61	5.00	0.23	0.00	0.23
6.00	2.00	0.61	5.00	0.23	0.00	0.23
6.05	2.00	0.61	5.00	0.23	0.00	0.23
6.10	2.00	0.61	5.00	0.23	0.00	0.23
6.15	2.00	0.61	5.00	0.23	0.00	0.23
6.20	2.00	0.61	5.00	0.23	0.00	0.23
6.25	2.00	$\substack{0.61\\0.61}$	5.00 5.00	0.23 0.23	0.00	0.23 0.23
6.30 6.35	2.00 2.00	0.61	5.00	0.23	$0.00 \\ 0.00$	0.23
6.40	2.00	0.61	5.00	0.23	0.00	0.23
6.45	2.00	0.61	5.00	0.23	0.00	0.23
6.50	2.00	0.61	5.00	0.23	0.00	0.23
6.55	2.00	0.61	5.00	0.23	0.00	0.23
6.60	2.00	0.61	5.00	0.23	0.00	0.23
6.65	2.00	0.61	5.00	0.23	0.00	0.23
6.70	2.00	0.61	5.00	0.23	0.00	0.23
6.75	2.00	0.61	5.00	0.23	0.00	0.23
6.80	2.00	0.61	5.00	0.23	0.00	0.23
6.85	2.00	0.61	5.00	0.23	0.00	0.23
6.90	2.00	0.61	5.00	0.23 0.23	0.00	0.23
6.95 7.00	2.00	0.61 0.61	5.00 5.00	0.23	$0.00 \\ 0.00$	0.23 0.23
7.05	2.00	0.61	5.00	0.23	0.00	0.23
7.10	2.00	0.61	5.00	0.23	0.00	0.23
7.15	2.00	0.61	5.00	0.23	0.00	0.23
7.20	2.00	0.61	5.00	0.23	0.00	0.23
7.25	2.00	0.61	5.00	0.23	0.00	0.23
7.30	2.00	0.61	5.00	0.23	0.00	0.23
7.35	2.00	0.61	5.00	0.23	0.00	0.23
7.40	2.00	0.61	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
7.45 7.50	2.00 2.00	0.61 0.61	5.00	0.23	0.00	0.23
7.55	2.00	0.61	5.00	0.23	0.00	0.23
7.60	2.00	0.61	5.00	0.23	0.00	0.23
7.65	2.00	0.61	5.00	0.23	0.00	0.23
7.70	2.00	0.61	5.00	0.23 0.23	0.00	0.23
7.75	2.00	0.61	5.00	0.23	0.00	0.23
7.80	2.00	0.61	5.00	0.23	0.00	0.23
7.85	2.00	0.61	5.00	0.23 0.23	0.00	0.23 0.23
7.90 7.95	2.00	0.61 0.61	5.00 5.00	0.23	$0.00 \\ 0.00$	0.23
8.00	2.00	0.61	5.00	0.23	0.00	0.23
8.05	2.00	0.61	5.00	0.23 0.23	0.00	0.23 0.23
8.10	2.00	0.61	5.00	0.23	0.00	0.23
8.15	2.00	0.61	5.00	0.23 0.23	0.00	0.23 0.23
8.20	2.00	0.61	5.00	0.23	0.00	0.23
8.25	2.00	0.61	5.00	0.23	0.00	0.23
8.30	2.00	0.61	5.00	0.23 0.23	0.00	0.23 0.23
8.35	2.00	0.61	5.00	0.23	0.00	0.23
8.40 8.45	2.00 2.00	0.61 0.61	5.00 5.00	0.23	$0.00 \\ 0.00$	0.23
8.50	2.00	0.61	5.00	0.23 0.23	0.00	0.23 0.23
8.55	2.00	0.61	5.00	0.23	0.00	0.23
8.60	2.00	0.61	5.00	0.23	0.00	0.23
8.65	2.00	0.61	5.00	0.23	0.00	0.23
				Dago 1		

		Santa To	reca HC	class Co	ncession	FR4 SIIM
8.70	2.00	0.61	5.00	0.23	0.00	0.23
8.75	2.00	0.61	5.00	0.23	0.00	0.23
8.80	2.00	0.61	5.00	0.23	0.00	0.23
	2.00	0.61	5.00	0.23	0.00	0.23
8.85 8.90		0.61	5.00	0.23	0.00	0.23
	2.00			0.23		0.23
8.95	2.00	0.61	5.00		0.00	
9.00	2.00	0.61	5.00	0.23	0.00	0.23
9.05	2.00	0.61	5.00	0.23	0.00	0.23
9.10	2.00	0.61	5.00	0.23	0.00	0.23
9.15	2.00	0.61	5.00	0.23	0.00	0.23
9.20	2.00	0.61	5.00	0.23	0.00	0.23
9.25	2.00	0.61	5.00	0.23	0.00	0.23
9.30	2.00	0.61	5.00	0.23	0.00	0.23
9.35	2.00	0.61	5.00	0.23	0.00	0.23
9.40	2.00	0.61	5.00	0.23	0.00	0.23
9.45	2.00	0.61	5.00	0.23	0.00	0.23
9.50	2.00	0.61	5.00	0.23	0.00	0.23
9.55	2.00	0.61	5.00 5.00	0.23	0.00	0.23
9.60	2.00	0.61	5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
9.65	2.00	$\substack{0.61\\0.61}$		0.23	0.00	0.23
9.70	2.00		5.00			0.23
9.75	2.00	0.61	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
9.80 9.85	2.00	0.61 0.61	5.00	0.23	0.00	0.23
9.90	2.00	0.61	5.00	0.23	0.00	0.23
9.90	2.00	0.61	5.00	0.23	0.00	0.23
10.00	2.00	0.61	5.00	0.23	0.00	0.23
10.05	2.00	0.61	5.00	0.23	0.00	0.23
10.10	2.00	0.61	5.00	0.23	0.00	0.23
10.15	2.00	0.61	5.00	0.23	0.00	0.23
10.20	2.00	0.61	5.00	0.23	0.00	0.23
10.25	2.00	0.61	5.00	0.23	0.00	0.23
10.30	2.00	0.62	5.00	0.23	0.00	0.23
10.35	2.00	0.62	5.00	0.23	0.00	0.23
10.40	2.00	0.62	5.00	0.23	0.00	0.23
10.45	2.00	0.62	5.00	0.23	0.00	0.23
10.50	2.00	0.62	5.00	0.23	0.00	0.23
10.55	2.00	0.62	5.00	0.23	0.00	0.23
10.60	2.00	0.63	5.00	0.23	0.00	0.23
10.65	2.00	0.63	5.00	0.23	0.00	0.23
10.70	2.00	0.63	5.00	0.23	0.00	0.23
10.75	2.00	0.63	5.00	0.23	0.00	0.23
10.80	2.00	0.63	5.00	0.23	0.00	0.23
10.85	2.00	0.63	5.00	0.23	0.00	0.23
10.90	2.00	0.63	5.00	0.23	0.00	0.23
10.95	2.00	0.64	5.00	0.23	0.00	0.23
11.00	2.00	0.64	5.00	0.23	0.00	0.23
11.05	2.00	0.64	5.00	0.23	0.00	0.23
11.10	2.00	0.64	5.00	0.23	0.00	0.23
11.15	2.00	0.64	5.00	0.23	0.00	0.23
11.20	2.00	0.64	5.00	0.23	0.00	0.23
11.25	2.00	0.64 0.65	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
$11.30 \\ 11.35$	2.00 2.00	0.65	5.00	0.23	0.00	0.23
11.40	2.00	0.65	5.00	0.23	0.00	0.23
11.45	2.00	0.65	5.00	0.23	0.00	0.23
11.50	2.00	0.65	5.00	0.23	0.00	0.23
11.55	2.00	0.65	5.00	0.23	0.00	0.23
11.60	2.00	0.65	5.00	0.23	0.00	0.23
11.65	2.00	0.66	5.00	0.23	0.00	0.23
11.70	2.00	0.66	5.00	0.23	0.00	0.23
11.75	2.00	0.66	5.00	0.23	0.00	0.23
11.80	2.00	0.66	5.00	0.23	0.00	0.23

		C T-	116	67 6		5D4
11.85	2.00	0.66	5.00	Class Cor 0.23	0.00	EB4.sum 0.23
11.90	2.00	0.66	5.00	0.23	0.00	0.23
11.95	2.00	0.66	5.00	0.23	0.00	0.23
12.00	2.00	0.67	5.00	0.23	0.00	0.23
12.05	2.00	0.67	5.00	0.23	0.00	0.23
12.10	2.00	0.67	5.00	0.23	0.00	0.23
12.15	2.00	0.67	5.00	0.23	0.00	0.23
12.20	2.00	0.67	5.00	0.23	0.00	0.23
12.25	2.00	0.67	5.00	0.23 0.23	0.00	0.23
12.30	2.00	0.67	5.00	0.23	0.00	0.23
12.35	2.00	0.67	5.00	0.23	0.00	0.23
12.40	2.00	0.68	5.00	0.23	0.00	0.23
12.45	2.00	0.68	5.00	0.23	0.00	0.23
12.50	2.00	0.68	5.00	0.23 0.23	0.00	0.23
12.55	2.00	0.68	5.00	0.23	0.00	0.23
12.60	2.00	0.68	5.00	0.23	0.00	0.23
12.65	2.00	0.68	5.00	0.23	0.00	0.23
12.70	2.00	0.68	5.00	0.23	0.00	0.23
12.75 12.80	2.00 2.00	0.68 0.69	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
12.85	2.00	0.69	5.00	0.23	0.00	0.23
12.90	2.00	0.69	5.00	0.23	0.00	0.23
12.95	2.00	0.69	5.00	0.23	0.00	0.23
13.00	2.00	0.69	5.00	0.23	0.00	0.23
13.05	2.00	0.69	5.00	0.23	0.00	0.23
13.10	2.00	0.69	5.00	0.23	0.00	0.23
13.15	2.00	0.69	5.00	0.23	0.00	0.23
13.20	2.00	0.69	5.00	0.23	0.00	0.23
13.25	2.00	0.70	5.00	0.23 0.23	0.00	0.23 0.23
$13.30 \\ 13.35$	2.00	0.70 0.70	5.00 5.00	0.23	0.00 0.00	0.23
13.40	2.00	0.70	5.00	0.23	0.00	0.23
13.45	2.00	0.70	5.00	0.23	0.00	0.23
13.50	2.00	0.70	5.00	0.23	0.00	0.23
13.55	2.00	0.70	5.00	0.23	0.00	0.23
13.60	2.00	0.70	5.00	0.23	0.00	0.23
13.65	2.00	0.71	5.00	0.23	0.00	0.23
13.70	2.00	0.71	5.00	0.23	0.00	0.23
13.75	2.00	0.71	5.00	0.23	0.00	0.23
13.80	2.00	0.71	5.00	0.23	0.00	0.23
$13.85 \\ 13.90$	2.00	$0.71 \\ 0.71$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
13.95	2.00	0.71	5.00	0.23	0.00	0.23
14.00	2.00	0.71	5.00	0.23	0.00	0.23
14.05	2.00	0.71	5.00	0.23 0.23	0.00	0.23
14.10	2.00	0.72	5.00	0.23	0.00	0.23
14.15	2.00	0.72	5.00	0.23 0.23	0.00	0.23 0.23
14.20	2.00	0.72	5.00	0.23	0.00	0.23
14.25	2.00	0.72	5.00	0.23	0.00	0.23
14.30	2.00	0.72	5.00	0.23 0.23	0.00	0.23 0.23
14.35	2.00	0.72	5.00	0.23	0.00	0.23
14.40 14.45	2.00	0.72 0.72	5.00 5.00	0.23	0.00 0.00	0.23
14.50	2.00	0.72	5.00	0.23 0.23	0.00	0.23
14.55	2.00	0.72	5.00	0.23	0.00	0.23
14.60	2.00	0.73	5.00	0.23	0.00	0.23
14.65	2.00	0.73	5.00	0.23 0.23	0.00	0.23 0.23
14.70	2.00	0.73	5.00	0.23	0.00	0.23
14.75	2.00	0.73	5.00	0.23 0.23	0.00	0.23
14.80	2.00	0.73	5.00	0.23	0.00	0.23
14.85 14.90	2.00	0.73 0.73	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23
14.95	2.00	0.73	5.00	0.23	0.00	0.23 0.23
11.55	2.00	0.75	3.00	D. 23	0.00	0.23

				class Cor		
15.00 15.05	2.00 2.00	0.73 0.73	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
15.10	2.00	0.74	5.00	0.23	0.00	0.23
15.15	2.00	0.74	5.00	0.23	0.00	0.23
15.20 15.25	2.00 2.00	0.74 0.74	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
15.30	2.00	0.74	5.00	0.23	0.00	0.23
15.35	2.00	0.74	5.00	0.23	0.00	0.23
15.40	2.00	0.74	5.00	0.23	0.00	0.23
15.45 15.50	2.00	0.74 0.74	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
15.55	2.00	0.74	5.00	0.23	0.00	0.23
15.60	2.00	0.75	5.00	0.23	0.00	0.23
15.65 15.70	2.00 2.00	0.75 0.75	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
15.75	2.00	0.75	5.00	0.23	0.00	0.23
15.80	2.00	0.75	5.00	0.23	0.00	0.23
15.85 15.90	2.00	0.75 0.75	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
15.95	2.00	0.75	5.00	0.23	0.00	0.23
16.00	2.00	0.75	5.00 5.00	0.23	0.00	0.23
16.05 16.10	2.00	0.75 0.75	5.00	0.23 0.23	0.00 0.00	0.23 0.23
16.15	2.00	0.76	5.00	0.23	0.00	0.23
16.20	2.00	0.76	5.00 5.00	0.23 0.23	0.00	0.23 0.23
16.25 16.30	2.00	0.76 0.76	5.00	0.23	0.00 0.00	0.23
16.35	2.00	0.76	5.00	0.23	0.00	0.23
16.40 16.45	2.00	0.76 0.76	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
16.50	2.00	0.76	5.00	0.23	0.00	0.23
16.55	2.00	0.76	5.00	0.23	0.00	0.23
16.60 16.65	2.00	0.76 0.76	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
16.70	2.00	0.77	5.00	0.23	0.00	0.23
16.75	2.00	0.77	5.00	0.23	0.00	0.23
16.80 16.85	2.00	0.77 0.77	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
16.90	2.00	0.77	5.00	0.23	0.00	0.23
16.95	2.00	0.77	5.00	0.23	0.00	0.23 0.23
17.00 17.05	2.00	0.77 0.77	5.00 5.00	0.23 0.23	0.00 0.00	0.23
17.10	2.00	0.77	5.00	0.23	0.00	0.23
17.15 17.20	2.00	0.77 0.77	5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
17.25	2.00	0.77	5.00 5.00	0.23	0.00	0.23
17.30 17.35	2.00	0.78	5.00	0.23	0.00	0.23
17.35 17.40	2.00	0.78 0.78	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
17.45	2.00	0.78	5.00	0.23	0.00	0.23
17.50	2.00	0.78	5.00	0.23	0.00	0.23
17.55 17.60	2.00 2.00	0.78 0.78	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
17.65	2.00	0.78	5.00	0.23	0.00	0.23
17.70	2.00	0.78	5.00	0.23	0.00	0.23
17.75 17.80	2.00	0.78 0.78	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
17.85	2.00	0.78	5.00	0.23	0.00	0.23
17.90 17.95	2.00	0.79 0.79	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
18.00	2.00	0.79	5.00	0.23	0.00	0.23
18.05	2.00	0.79	5.00	0.23	0.00	0.23
18.10	2.00	0.79	5.00	0.23	0.00	0.23

10.15	2 00			Class Cor		
18.15 18.20	2.00	0.79 0.79	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
18.25 18.30	2.00	0.79 0.79	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
18.35	2.00	0.79	5.00	0.23	0.00	0.23
18.40 18.45	2.00 2.00	0.79 0.79	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
18.50	2.00	0.80	5.00	0.23	0.00	0.23
18.55 18.60	2.00	0.80 0.80	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
18.65 18.70	2.00	0.80	5.00	0.23 0.23	0.00	0.23 0.23
18.75	2.00	$0.80 \\ 0.80$	5.00 5.00	0.23	0.00	0.23
18.80 18.85	2.00	0.80 0.80	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
18.90	2.00	0.80	5.00	0.23	0.00	0.23
18.95 19.00	2.00 2.00	0.80 0.80	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
$19.05 \\ 19.10$	2.00	$0.80 \\ 0.80$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
19.15	2.00	0.81	5.00	0.23	0.00	0.23
19.20 19.25	2.00	$\substack{0.81\\0.81}$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
19.30	2.00	0.81	5.00	0.23	0.00	0.23
19.35 19.40	2.00	$\substack{0.81\\0.81}$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
19.45 19.50	2.00 2.00	$\substack{0.81\\0.81}$	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
19.55	2.00	0.81	5.00	0.23	0.00	0.23
19.60 19.65	2.00	$\substack{0.81\\0.81}$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
19.70 19.75	2.00	0.81 0.81	5.00 5.00	0.23 0.23	0.00	0.23 0.23
19.80	2.00	0.82	5.00	0.23	0.00	0.23
$19.85 \\ 19.90$	2.00 2.00	0.82 0.82	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
19.95	2.00	0.82	5.00	0.23	0.00	0.23
20.00 20.05	2.00	0.82 0.82	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
20.10 20.15	2.00	0.82 0.82	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23
20.20	2.00	0.82	5.00	0.23	0.00	0.23
20.25 20.30	2.00	0.82 0.82	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
20.35 20.40	2.00	0.82 0.82	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
20.45	2.00	0.82	5.00	0.23	0.00	0.23
20.50 20.55	2.00	0.83 0.83	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
20.60	2.00	0.83	5.00	0.23	0.00	0.23
20.65 20.70	2.00	0.83 0.83	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
20.75 20.80	2.00	0.83 0.83	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
20.85	2.00	0.83	5.00	0.23	0.00	0.23
20.90 20.95	2.00	0.83 0.83	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
21.00 21.05	2.00	0.83 0.83	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
21.10	2.00	0.83	5.00	0.23	0.00	0.23
21.15 21.20	2.00	0.83 0.83	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
21.25	2.00	0.84	5.00	0.23	0.00	0.23

		Santa Te		Class Co	ncession	
21.30 21.35	2.00 2.00	0.84 0.84	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
21.40	2.00	0.84	5.00	0.23	0.00	0.23
21.45	2.00	0.84	5.00	0.23	0.00	0.23
21.50 21.55	2.00 2.00	0.84 0.84	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
21.60	2.00	0.84	5.00	0.23	0.00	0.23
21.65 21.70	2.00	0.84 0.84	5.00	0.23 0.23	0.00 0.00	0.23 0.23
21.75	2.00	0.84	5.00 5.00	0.23	0.00	0.23
21.80	2.00	0.84	5.00	0.23	0.00	0.23
$21.85 \\ 21.90$	2.00 2.00	0.84 0.84	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
21.95	2.00	0.84	5.00	0.23	0.00	0.23
22.00	2.00 2.00	0.85	5.00 5.00	0.23 0.23	0.00	0.23 0.23
22.05 22.10	2.00	$\begin{array}{c} 0.85 \\ 0.85 \end{array}$	5.00	0.23	0.00 0.00	0.23
22.15	2.00	0.85	5.00	0.23	0.00	0.23
22.20	2.00	0.85 0.85	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
22.25 22.30	2.00	0.85	5.00	0.23	0.00	0.23
22.35	2.00	0.85	5.00	0.23	0.00	0.23
22.40 22.45	2.00 2.00	0.85 0.85	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
22.50	2.00	0.85	5.00	0.23	0.00	0.23
22.55 22.60	2.00	0.85 0.85	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
22.65	2.00	0.85	5.00	0.23	0.00	0.23
22.70 22.75	2.00	0.85 0.85	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23
22.80	2.00	0.85	5.00	0.23	0.00	0.23
22.85 22.90	2.00 2.00	$0.86 \\ 0.86$	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
22.95	2.00	0.86	5.00	0.23	0.00	0.23
23.00	2.00	0.86	5.00	0.23	0.00	0.23
23.05 23.10	2.00 2.00	$0.86 \\ 0.86$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
23.15	2.00	0.86	5.00	0.23	0.00	0.23
23.20 23.25	2.00 2.00	0.86 0.86	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
23.30	2.00	0.86	5.00	0.23	0.00	0.23
23.35 23.40	2.00	0.86 0.86	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
23.45	2.00	0.86	5.00	0.23	0.00	0.23
23.50	2.00	0.86	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
23.55 23.60	2.00	0.86 0.86	5.00	0.23	0.00	0.23
23.65	2.00	0.86	5.00	0.23	0.00	0.23
23.70 23.75	2.00	0.86 0.87	5.00 5.00	0.23 0.23	0.00 0.00	0.23
23.80	2.00	0.87	5.00	0.23	0.00	0.23
23.85 23.90	2.00 2.00	0.87 0.87	5.00 5.00	0.23 0.23	0.00 0.00	0.23
23.95	2.00	0.87	5.00	0.23	0.00	0.23 0.23
24.00 24.05	2.00	0.87 0.87	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
24.10	2.00	0.87	5.00	0.23	0.00	0.23
24.15	2.00	0.87	5.00	0.23	0.00	0.23
24.20 24.25	2.00	0.87 0.87	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
24.30	2.00	0.87	5.00	0.23	0.00	0.23
24.35 24.40	2.00 2.00	0.87 0.87	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23
				Dage 0		

24.450 24.555 24.665 24.750 24.750 24.885 24.895 25.305 25.305 25.305 25.305 25.305 26	2.00 2.00 2.00 2.00 2.00 2.00 2.00 2.00	0.87 0.87 0.87 0.887 0.887 0.888 0.889 0.899 0.890 0.890 0.890 0.890 0.890 0.890 0.890 0.890 0.890 0.890 0.8	5.00 5.00	Class Con 0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23	0.00 0.00	0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23
26.00 26.05 26.10 26.15 26.20 26.30 26.35 26.40 26.55 26.60 26.55 26.65 26.70 26.85 26.80 26.95 27.00 27.10 27.15 27.20 27.30	2.00 2.00 2.00 2.00 2.00 2.00 2.00 2.00	0.89 0.89 0.89 0.89 0.89 0.89 0.89 0.89	5.00 5.00 5.00 5.00 5.00 5.00 5.00 5.00	0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	0.23 0.23 0.23 0.23 0.23 0.23 0.23 0.23
27.35 27.40 27.45 27.50 27.55	2.00 2.00 2.00 2.00 2.00	0.90 0.90 0.90 0.90 0.90	5.00 5.00 5.00 5.00 5.00	0.23 0.23 0.23 0.23 0.23 Page 10	0.00 0.00 0.00 0.00 0.00	0.23 0.23 0.23 0.23 0.23

		Santa Te	resa HS	class Cor	ncession	EB4.sum
27.60	2.00	0.90	5.00	0.23	0.00	0.23
27.65	2.00	0.90	5.00	0.23	0.00	0.23
27.70	2.00	0.90	5.00	0.23	0.00	0.23
27.75	2.00	0.90	5.00	0.23	0.00	0.23
27.80	2.00	0.90	5.00	0.23 0.23	0.00	0.23 0.23
27.85 27.90	2.00 2.00	0.90 0.90	5.00 5.00	0.23	$0.00 \\ 0.00$	0.23
27.95	2.00	0.90	5.00	0.23	0.00	0.23
28.00	2.00	0.90	5.00	0.23	0.00	0.23
28.05	2.00	0.90	5.00	0.23	0.00	0.23
28.10	2.00	0.90	5.00	0.23	0.00	0.23
28.15	2.00	0.90	5.00	0.23	0.00	0.23
28.20	2.00	0.90	5.00	0.23	0.00	0.23
28.25	2.00	0.90	5.00	0.23 0.23	0.00	0.23 0.23
28.30 28.35	2.00	$\substack{0.91\\0.91}$	5.00 5.00	0.23	$0.00 \\ 0.00$	0.23
28.40	2.00	0.91	5.00	0.23	0.00	0.23
28.45	2.00	0.91	5.00	0.23	0.00	0.23
28.50	2.00	0.91	5.00	0.23	0.00	0.23
28.55	2.00	0.91	5.00	0.23	0.00	0.23
28.60	2.00	0.91	5.00	0.23	0.00	0.23
28.65 28.70	2.00 2.00	$0.91 \\ 0.91$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
28.75	2.00	0.91	5.00	0.23	0.00	0.23
28.80	2.00	0.91	5.00	0.23	0.00	0.23
28.85	2.00	0.91	5.00	0.23	0.00	0.23
28.90	2.00	0.91	5.00	0.23	0.00	0.23
28.95	2.00	0.91	5.00	0.23	0.00	0.23
29.00	2.00	$0.91 \\ 0.91$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
29.05 29.10	2.00 2.00	0.91	5.00	0.23	0.00	0.23
29.15	2.00	0.91	5.00	0.23	0.00	0.23
29.20	2.00	0.91	5.00	0.23	0.00	0.23
29.25	2.00	0.91	5.00	0.23	0.00	0.23
29.30	2.00	0.91	5.00	0.23	0.00	0.23
29.35 29.40	2.00	$\substack{0.91\\0.91}$	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
29.45	2.00	$0.91 \\ 0.91$	5.00	0.23	0.00	0.23
29.50	2.00	0.91	5.00	0.23	0.00	0.23
29.55	2.00	0.91	5.00	0.23	0.00	0.23
29.60	2.00	0.91	5.00	0.23	0.00	0.23
29.65	2.00	0.91	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
29.70 29.75	2.00 2.00	0.91 0.92	5.00	0.23	0.00	0.23
29.80	2.00	0.92	5.00	0.23	0.00	0.23
29.85	2.00	0.92	5.00	0.23	0.00	0.23
29.90	2.00	0.92	5.00	0.23	0.00	0.23
29.95	2.00	0.92	5.00	0.23	0.00	0.23
30.00	2.00	0.92 0.92	5.00 5.00	0.23 0.23	0.00	0.23 0.23
30.05 30.10	2.00 2.00	0.92	5.00	0.23	$0.00 \\ 0.00$	0.23
30.15	2.00	0.92	5.00	0.23	0.00	0.23
30.20	2.00	0.92	5.00	0.23	0.00	0.23
30.25	2.00	0.92	5.00	0.23	0.00	0.23
30.30	2.00	0.92	5.00	0.23	0.00	0.23
30.35 30.40	2.00 2.00	0.92 0.92	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
30.45	2.00	0.92	5.00	0.23	0.00	0.23
30.50	2.00	0.92	5.00	0.23	0.00	0.23
30.55	2.00	0.92	5.00	0.23	0.00	0.23
30.60	2.00	0.92	5.00	0.23	0.00	0.23
30.65	2.00	0.92	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
30.70	2.00	0.92	5.00	0.23 Page 11	0.00	0.23

		Santa Te	resa HS	Class Con	cession	EB4.sum
30.75	2.00	0.92	5.00	0.23	0.00	0.23
30.80	2.00	0.92	5.00	0.23	0.00	0.23
30.85 30.90	2.00	0.92 0.92	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
30.95	2.00	0.92	5.00	0.23	0.00	0.23
31.00	2.00	0.92	5.00	0.23	0.00	0.23
31.05	2.00	0.92	5.00	0.23	0.00	0.23
31.10 31.15	2.00	0.92 0.92	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
31.20	2.00	0.92	5.00	0.23	0.00	0.23
31.25	2.00	0.92	5.00	0.23	0.00	0.23
31.30 31.35	2.00	0.92 0.92	5.00 5.00	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
31.40	2.00	0.92	5.00	0.23	0.00	0.23
31.45	2.00	0.92	5.00	0.23	0.00	0.23
31.50 31.55	2.00	0.92 0.92	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
31.60	2.00	0.92	5.00	0.23	0.00	0.23
31.65	2.00	0.92	5.00	0.23	0.00	0.23
31.70 31.75	2.00	0.92 0.92	5.00 5.00	0.23 0.23	0.00 0.00	0.23 0.23
31.80	2.00	0.92	5.00	0.23	0.00	0.23
31.85	2.00	0.92	5.00	0.23	0.00	0.23
31.90	2.00	0.92	5.00 5.00	0.23	$0.00 \\ 0.00$	0.23 0.23
31.95 32.00	2.00	0.92 0.92	5.00	0.23 0.23	0.00	0.23
32.05	1.45	0.92	1.58	0.23	0.00	0.23
32.10	1.45	0.92	1.58	0.23 0.23	0.00	0.23
32.15 32.20	$\frac{1.45}{1.45}$	0.92 0.92	$\frac{1.58}{1.58}$	0.23	$0.00 \\ 0.00$	0.23 0.23
32.25	1.45	0.92	1.58	0.23	0.00	0.23
32.30	1.45	0.92	1.59	0.23	0.00	0.23
32.35 32.40	1.45 1.45	0.92 0.92	$1.59 \\ 1.59$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
32.45	1.45	0.92	1.59	0.23	0.00	0.23
32.50	1.45	0.92	1.59	0.23	0.00	0.23
32.55 32.60	1.45 1.45	0.92 0.92	$1.59 \\ 1.59$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
32.65	1.45	0.92	1.59	0.23	0.00	0.23
32.70	1.45	0.91	1.59	0.23	0.00	0.23
32.75 32.80	$\frac{1.45}{1.45}$	$\substack{0.91\\0.91}$	$\frac{1.59}{1.59}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
32.85	1.45	0.91	1.59	0.23	0.00	0.23
32.90	1.45	0.91	1.59	0.23	0.00	0.23
32.95 33.00	1.45 1.45	$\substack{0.91\\0.91}$	$\frac{1.59}{1.59}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
33.05	1.45	0.91	1.59	0.23	0.00	0.23
33.10	1.45	0.91	1.59	0.23	0.00	0.23
33.15 33.20	$\frac{1.45}{1.45}$	$\substack{0.91\\0.91}$	$\frac{1.59}{1.59}$	0.23 0.23	0.00 0.00	0.23 0.23
33.25	1.45	0.91	1.59	0.23	0.00	0.23
33.30	1.45	0.91	1.59	0.23 0.23	0.00	0.23
33.35	1.45 1.45	$\substack{0.91\\0.91}$	1.59 1.59	0.23	0.00 0.00	0.23 0.23
33.40 33.45	1.45	$0.91 \\ 0.91$	1.59	0.23	0.00	0.23
33.50	1.45	0.91	1.59	0.23 0.23	0.00	0.23
33.55	1.45	0.91	$\frac{1.59}{1.50}$	0.23	$0.00 \\ 0.00$	0.23
33.60 33.65	$\frac{1.45}{1.45}$	$\substack{0.91\\0.91}$	$\frac{1.59}{1.59}$	0.23 0.23	0.00	0.23 0.23
33.70	1.45	0.91	1.59	0.23	0.00	0.23
33.75	1.45	0.91	$\frac{1.59}{1.59}$	0.23 0.23	0.00	0.23 0.23
33.80 33.85	$\frac{1.45}{1.45}$	$0.91 \\ 0.91$	1.59	0.23	0.00 0.00	0.23
				Daga 13		

		Santa Te	eresa HS	Class Cor	ncession	EB4.sum
33.90	1.45	0.91	1.59	0.23	0.00	0.23
33.95	1.45	0.91	1.59	0.23	0.00	0.23
34.00	1.45	0.91	1.59	0.23	0.00	0.23
34.05 34.10	$\frac{1.45}{1.45}$	$0.91 \\ 0.91$	$1.59 \\ 1.59$	0.23 0.23	0.00 0.00	0.23 0.23
34.15	1.45	0.91	1.59	0.23	0.00	0.23
34.20	1.45	0.91	1.59	0.23	0.00	0.23
34.25	1.45	0.91	1.59	0.23	0.00	0.23
34.30	1.45	0.91	1.59	0.23	0.00	0.23
34.35	$\frac{1.45}{1.45}$	0.91	1.59	0.23 0.23	0.00	0.23 0.23
34.40 34.45	1.45	$\begin{array}{c} 0.91 \\ 0.91 \end{array}$	$1.59 \\ 1.59$	0.23	$0.00 \\ 0.00$	0.23
34.50	1.45	0.91	1.59	0.23	0.00	0.23
34.55	1.45	0.91	1.59	0.23	0.00	0.23
34.60	1.45 1.45	$0.91 \\ 0.91$	$\frac{1.59}{1.59}$	0.23 0.23	0.00	0.23 0.23
34.65 34.70	1.45	$0.91 \\ 0.91$	1.59	0.23	0.00 0.00	0.23
34.75	1.45	0.91	1.59	0.23	0.00	0.23
34.80	1.45	0.91	1.59	0.23	0.00	0.23
34.85	1.45	0.91	1.59	0.23	0.00	0.23
34.90 34.95	$\frac{1.45}{1.45}$	$0.91 \\ 0.91$	$1.59 \\ 1.59$	0.23 0.23	0.00 0.00	0.23 0.23
35.00	1.45	$0.91 \\ 0.91$	1.60	0.23	0.00	0.23
35.05	1.45	0.91	1.60	0.23	0.00	0.23
35.10	1.45	0.91	1.60	0.23	0.00	0.23
35.15 35.20	$\frac{1.45}{1.45}$	$0.91 \\ 0.91$	$1.60 \\ 1.60$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
35.25	1.45	0.91	1.60	0.23	0.00	0.23
35.25 35.30	1.45	0.91	1.60	0.23	0.00	0.23
35.35	1.45	0.91	1.60	0.23	0.00	0.23
35.40 35.45	$1.45 \\ 1.45$	$\begin{array}{c} 0.91 \\ 0.91 \end{array}$	$1.60 \\ 1.60$	0.23 0.23	0.00 0.00	0.23 0.23
35.50	1.45	0.91	1.60	0.23	0.00	0.23
35.55	1.45	0.91	1.60	0.23	0.00	0.23
35.60	1.45	0.91	1.60	0.23	0.00	0.23
35.65 35.70	1.45 1.45	$0.91 \\ 0.91$	$\frac{1.60}{1.60}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
35.75	1.45	0.91	1.60	0.23	0.00	0.23
35.80	1.45	0.91	1.60	0.23	0.00	0.23
35.85	1.45	0.91	1.60	0.23	0.00	0.23
35.90 35.95	$1.45 \\ 1.45$	$\substack{0.91\\0.91}$	$egin{array}{c} 1.60 \ 1.60 \end{array}$	0.23 0.23	0.00 0.00	0.23 0.23
36.00	1.45	0.91	1.60	0.23	0.00	0.23
36.05	1.45	0.91	1.60	0.23	0.00	0.23
36.10	1.45	0.91	1.60	0.23	0.00	0.23
36.15 36.20	1.45 1.45	$0.91 \\ 0.91$	$\substack{1.60\\1.60}$	0.23 0.23	0.00 0.00	0.23 0.23
36.25	1.45	0.91	1.60	0.23	0.00	0.23
36.25 36.30	1.45	0.91	1.60	0.23	0.00	0.23
36.35	1.45	0.91	$\frac{1.60}{1.60}$	0.23	0.00	0.23
36.40 36.45	$1.45 \\ 1.45$	$\substack{0.91\\0.91}$	$\substack{1.60\\1.60}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
36.50	1.45	0.91	1.60	0.23	0.00	0.23
36.55	1.45	0.90	1.60	0.23	0.00	0.23
36.60	1.45	0.90	1.60	0.23	0.00	0.23
36.65 36.70	1.45 1.45	$0.90 \\ 0.90$	$\substack{1.60\\1.60}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
36.75	1.45	0.90	1.60	0.23	0.00	0.23
36.80	1.45	0.90	1.61	0.23	0.00	0.23
36.85	1.45	0.90	1.61	0.23	0.00	0.23
36.90 36.95	$1.45 \\ 1.45$	$0.90 \\ 0.90$	$\substack{1.61\\1.61}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
37.00	1.45	0.90	1.61	0.23	0.00	0.23
				Dog 12		

		Santa Te	resa HS	Class Cor	cession	
37.05 37.10	$1.45 \\ 1.45$	$0.90 \\ 0.90$	$\substack{1.61\\1.61}$	0.23 0.23	$0.00 \\ 0.00$	0.23 0.23
37.15	1.45	0.90	1.61	0.23	0.00	0.23
37.20 37.25	1.45 1.45	0.90 0.90	$\frac{1.61}{1.61}$	0.23 0.23	0.00 0.00	0.23 0.23
37.30	1.45	0.90	1.61	0.23	0.00	0.23
37.35 37.40	1.45 1.45	$0.90 \\ 0.90$	$\substack{1.61\\1.61}$	0.23 0.23	$0.00 \\ 0.00$	0.23
37.45	1.45	0.90	1.61	0.23	0.00	0.23
37.50 37.55	$1.45 \\ 1.45$	$0.90 \\ 0.90$	$\substack{1.61\\1.61}$	0.23 0.23	0.00 0.00	0.23 0.23
37.60	1.45	0.90	1.61	0.22	0.00	0.22 0.22
37.65 37.70	$\frac{1.45}{1.45}$	0.90 0.90	$\substack{1.61\\1.61}$	0.22 0.22	0.00 0.00	0.22
37.75	1.45	0.90	$\frac{1.61}{1.61}$	0.22 0.22	0.00	0.22 0.22
37.80 37.85	1.45 1.45	0.90 0.90	$\substack{1.61\\1.61}$	0.22	0.00 0.00	0.22
37.90	1.45	0.90	$\substack{1.61\\1.61}$	0.22 0.22	$0.00 \\ 0.00$	0.22 0.22
37.95 38.00	$1.45 \\ 1.45$	0.90 0.90	1.61	0.22	0.00	0.22
38.05 38.10	1.45 1.45	$0.90 \\ 0.90$	$\frac{1.61}{1.61}$	0.21 0.21	0.00 0.00	0.21 0.21
38.15	1.45	0.90	1.61	0.21	0.00	0.21
38.20 38.25	1.45 1.45	0.90 0.90	$\substack{1.61\\1.61}$	0.21 0.21	0.00 0.00	0.21 0.21
38.30	1.45	0.90	1.62	0.20	0.00	0.20
38.35 38.40	1.45 1.45	0.90 0.90	$\frac{1.62}{1.62}$	0.20 0.20	0.00 0.00	0.20 0.20
38.45	1.45	0.90	1.62	0.20	0.00	0.20
38.50 38.55	$\frac{1.45}{1.45}$	0.90 0.90	$1.62 \\ 1.62$	$\substack{0.19\\0.19}$	0.00 0.00	$0.19 \\ 0.19$
38.60	1.45	0.90	1.62	0.19	0.00	0.19
38.65 38.70	1.45 1.45	0.90 0.90	1.62 1.62	$\begin{array}{c} 0.18 \\ 0.18 \end{array}$	0.00 0.00	0.18 0.18
38.75 38.80	1.45 1.45	0.90 0.90	1.62 1.62	0.17 0.17	$0.00 \\ 0.00$	0.17 0.17
38.85	1.45	0.90	1.62	0.17	0.00	0.17
38.90 38.95	$0.33 \\ 0.31$	0.90 0.90	0.37* 0.35*	$\begin{array}{c} 0.16 \\ 0.16 \end{array}$	0.00 0.00	$\begin{array}{c} 0.16 \\ 0.16 \end{array}$
39.00	0.30	0.90	0.33*	0.15	0.00	0.15
39.05 39.10	0.29 0.28	0.90 0.90	0.32* 0.31*	$\substack{0.15\\0.14}$	0.00 0.00	0.15 0.14
39.15	0.27	0.89	0.31*	0.14	0.00	0.14
39.20 39.25	0.27 0.26	$0.89 \\ 0.89$	0.30* 0.29*	$\substack{0.13\\0.13}$	$0.00 \\ 0.00$	$0.13 \\ 0.13$
39.30	0.26	0.89	0.29* 0.29*	0.12	$0.00 \\ 0.00$	0.12 0.12
39.35 39.40	0.26 0.25	$0.89 \\ 0.89$	0.29*	$\begin{array}{c} \textbf{0.12} \\ \textbf{0.11} \end{array}$	0.00	0.11
39.45	0.25 0.24	$0.89 \\ 0.89$	0.28* 0.27*	$\substack{0.10\\0.10}$	0.00 0.00	$\substack{0.10\\0.10}$
39.50 39.55	0.24	0.89	0.27*	0.09	0.00	0.09
39.60	0.25 0.25	0.89 0.89	0.28* 0.28*	0.09 0.08	0.00 0.00	0.09 0.08
39.65 39.70	0.25	0.89	0.28*	0.07	0.00	0.07
39.75 39.80	0.26 0.27	$0.89 \\ 0.89$	0.29* 0.30*	0.07 0.06	0.00 0.00	0.07 0.06
39.85	0.28	0.89	0.32*	0.06	0.00	0.06
39.90 39.95	$0.31 \\ 1.45$	$0.89 \\ 0.89$	0.35* 1.63	0.05 0.05	0.00 0.00	0.05 0.05
40.00	1.45	0.89	1.63	0.04	0.00	0.04
40.05 40.10	$\frac{1.45}{1.45}$	$0.89 \\ 0.89$	1.63 1.63	0.04 0.04	0.00 0.00	0.04 0.04
40.15	1.45	0.89	1.63	0.03 Page 14	0.00	0.03

		Santa Te	resa HS	Class Co	ncession	EB4.sum
40.20	1.45	0.89	1.63	0.03	0.00	0.03
40.25	1.45	0.89	1.63	0.03	0.00	0.03
40.30	1.45	0.89	1.63	0.02	0.00	0.02
40.35	1.45	0.89	1.63	0.02	0.00	0.02
40.40	1.45	0.89	1.63	0.02	0.00	0.02
40.45	1.45	0.89	1.63	0.02	0.00	0.02
40.50 40.55	1.45 1.45	$0.89 \\ 0.89$	$\substack{1.63\\1.63}$	0.02 0.02	0.00 0.00	0.02 0.02
40.60	1.45	0.89	1.63	0.02	0.00	0.02
40.65	1.45	0.89	1.63	0.01	0.00	0.01
40.70	1.45	0.89	1.63	0.01	0.00	0.01
40.75	1.45	0.89	1.63	0.01	0.00	0.01
40.80	1.45	0.89	1.64	0.01	0.00	0.01
40.85	1.45	0.89	1.64	0.01	0.00	0.01
40.90	1.45	0.89	1.64	0.01	0.00	0.01
40.95	1.45	0.89	1.64	$\begin{smallmatrix}0.01\\0.01\end{smallmatrix}$	$0.00 \\ 0.00$	$\substack{0.01\\0.01}$
41.00 41.05	$1.45 \\ 1.45$	$0.89 \\ 0.89$	$1.64 \\ 1.64$	0.01	0.00	$0.01 \\ 0.01$
41.10	1.45	0.89	1.64	0.01	0.00	0.01
41.15	1.45	0.89	1.64	0.01	0.00	0.01
41.20	1.45	0.89	1.64	0.01	0.00	0.01
41.25	1.45	0.89	1.64	0.01	0.00	0.01
41.30	1.45	0.89	1.64	0.01	0.00	0.01
41.35	1.45	0.88	1.64	0.01	0.00	0.01
41.40	1.45	0.88	1.64	0.01	0.00	0.01
41.45	1.45 1.45	0.88 0.88	1.64 1.64	$\substack{0.01\\0.01}$	0.00 0.00	$\begin{smallmatrix}0.01\\0.01\end{smallmatrix}$
41.50 41.55	1.45	0.88	1.64	$0.01 \\ 0.01$	0.00	0.01
41.60	1.45	0.88	1.64	$0.01 \\ 0.01$	0.00	0.01
41.65	1.45	0.88	1.64	0.01	0.00	0.01
41.70	1.45	0.88	1.64	0.00	0.00	0.00
41.75	1.45	0.88	1.64	0.00	0.00	0.00
41.80	1.45	0.88	1.64	0.00	0.00	0.00
41.85	1.45	0.88	1.64	0.00	0.00	0.00
41.90 41.95	$\frac{1.45}{1.45}$	0.88	$1.65 \\ 1.65$	0.00 0.00	0.00 0.00	$0.00 \\ 0.00$
42.00	$1.45 \\ 1.45$	0.88 0.88	1.65	0.00	0.00	0.00
42.05	2.00	0.88	5.00	0.00	0.00	0.00
42.10	2.00	0.88	5.00	0.00	0.00	0.00
42.15	2.00	0.88	5.00	0.00	0.00	0.00
42.20	2.00	0.88	5.00	0.00	0.00	0.00
42.25	2.00	0.88	5.00	0.00	0.00	0.00
42.30	2.00	0.88	5.00	0.00	0.00	0.00
42.35 42.40	2.00 2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
42.45	2.00	0.88	5.00	0.00	0.00	0.00
42.50	2.00	0.88	5.00	0.00	0.00	0.00
42.55	2.00	0.88	5.00	0.00	0.00	0.00
42.60	2.00	0.88	5.00	0.00	0.00	0.00
42.65	2.00	0.88	5.00	0.00	0.00	0.00
42.70	2.00	0.88	5.00	0.00	0.00	0.00
42.75	2.00	0.88	5.00	0.00	0.00	0.00
42.80 42.85	2.00 2.00	0.88 0.88	5.00 5.00	0.00 0.00	0.00 0.00	0.00 0.00
42.90	2.00	0.88	5.00	0.00	0.00	0.00
42.95	2.00	0.88	5.00	0.00	0.00	0.00
43.00	2.00	0.88	5.00	0.00	0.00	0.00
43.05	2.00	0.88	5.00	0.00	0.00	0.00
43.10	2.00	0.88	5.00	0.00	0.00	0.00
43.15	2.00	0.88	5.00	0.00	0.00	0.00
43.20 43.25	2.00 2.00	0.88 0.87	5.00 5.00	0.00 0.00	$0.00 \\ 0.00$	0.00 0.00
43.23	2.00	0.87	5.00	0.00	0.00	0.00
13.30	2.00	0.07	3.00	Page 15	0.00	0.00

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Santa Teresa HS Class Concession EB4.sum
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5.00
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43.35
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44.85
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44.90
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44.95
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45.00
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* F.S.<1, Liquefaction Potential Zone (F.S. is limited to 5, CRR is limited to 2, CSR is limited to 2)

Units: Unit: qc, fs, Stress or Pressure = atm (1.0581tsf); Unit Weight = pcf; Depth = ft; Settlement = in.

```
1 atm (atmosphere) = 1 tsf (ton/ft2)
CRRM Cyclic resistance ratio from soils
CSRsf Cyclic stress ratio induced by a given earthquake (with user request factor of safety)
F.S. Factor of Safety against liquefaction, F.S.=CRRm/CSRsf
S_sat Settlement from saturated sands
S_dry Settlement from Unsaturated Sands
S_all Total Settlement from Saturated and Unsaturated Sands
NoLiq No-Liquefy Soils
```